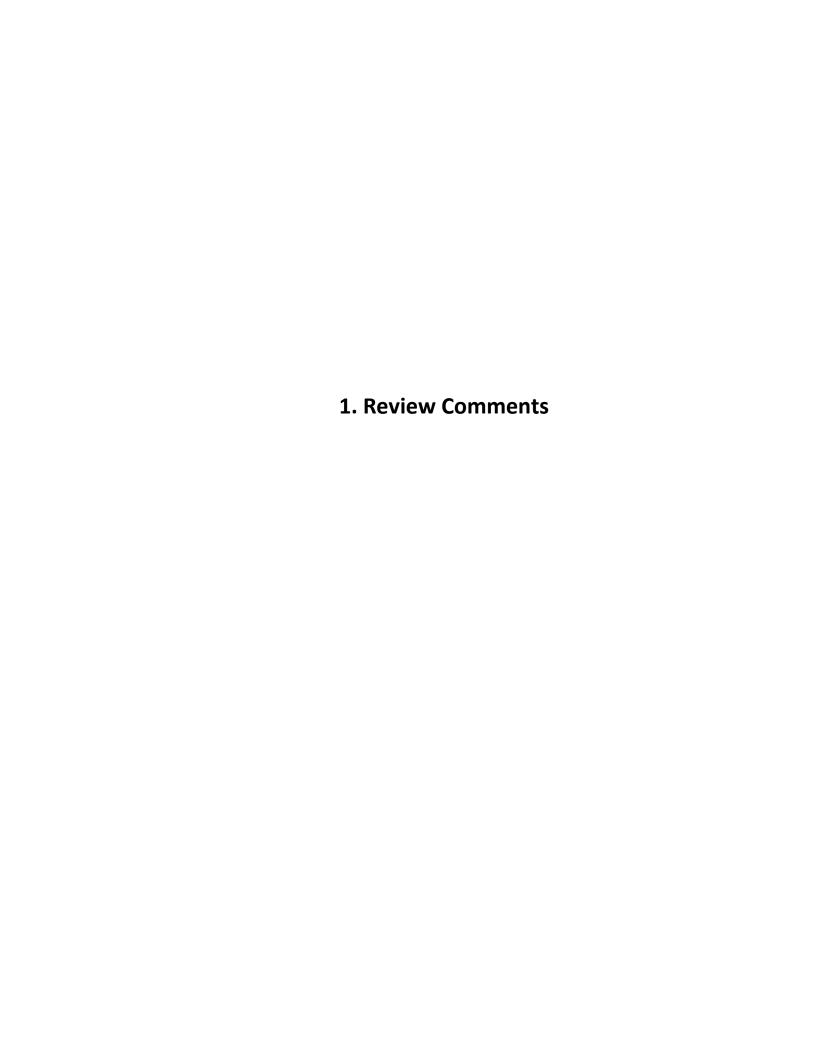
APPENDIX A

- **1. Review Comments**
- 2. Selection Process



Seth Asante

From: Raphael, Connie (DOT)

Sent: Wednesday, November 04, 2015 11:11 AM

To: Seth Asante

Subject: RE: Low-Cost Improvements to Express-Highway Bottleneck Locations

Hi Seth,

The District has reviewed the revised memo. Overall you put together a good explanation and plan for moving forward. Here are some comments.

Location 1: I-93 SB between I-95 and Montvale Ave appears to be the only one that could be implemented short-term, low-cost. Alternative 2, providing an auxiliary lane for the entire length between interchanges, is the better solution. The shoulder area appears to be wide enough to accommodate full-time travel and the emergency turnout would not be overly expensive to build. It would also match up well with the auxiliary lane on I-93 NB between the same interchanges. This solution would mirror the recently constructed northbound auxiliary lane and emergency pull off. The Design Exception Report required for the less than minimum right shoulder could be easily produced as it is similar to the DER submitted and approved for the northbound auxiliary lane.

Thanks Seth

Connie Raphael
District Four Planning Coordinator
MassDOT – Highway Division
519 Appleton Street
Arlington, MA 02476
781-641-8468

From: Seth Asante [mailto:sasante@ctps.org]
Sent: Wednesday, October 28, 2015 11:17 AM

To: Raphael, Connie (DOT)

Subject: Low-Cost Improvements to Express-Highway Bottleneck Locations

Hi Connie,

The attachment is a revised memo with Figures 2 and 9 legends corrected to AM hours.

Thanks, Seth

Seth A. Asante | Chief Transportation Planner CENTRAL TRANSPORTATION PLANNING STAFF 857.702.3644 | sasante@ctps.org www.ctps.org/bostonmpo

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Seth Asante

From: Lipton, Amitai (DOT)

Sent: Thursday, November 05, 2015 12:25 PM

To: Seth Asante (sasante@ctps.org)

Cc: Boudreau, Neil (DOT); Kulen, Raj (DOT); Pervez, Hameed (DOT)

Subject: RE: Low-Cost Improvements to Express-Highway Bottleneck Locations

Attachments: 20150114 Memo Heller to Leavenworth w-appendices.pdf; 20151007 Memo Heller to

Leavenworth - scanned signed by DHD.pdf

Hi Seth,

This is a very thorough analysis of the bottleneck at I-95 SB at I-90 in Weston. I'd like to second Raj's suggestion that it may be worthwhile to send your memo to FHWA ASAP (in draft version), as they are in the middle of evaluating our request to make the modifications permanent.

I do have a few minor notes/suggestions for your consideration:

Please note District 6 implemented the sign and pavement marking modifications in June 2015. Were the field observations and traffic volume measurements conducted before or after the modifications were implemented? In Table 5 (page 18), the existing conditions are labeled as "2015", and I was curious if these were counts actually conducted in 2015, or if they were older counts with a growth rate applied?

There seems to be a little inconsistency regarding speeds and terminology in certain parts of pages 14-15. The existing geometry was evaluated in reference to either a 65 mph (exit ramp to Route 30) or 70 mph (entrance ramps) design speed on I-95. While the posted speed limit on I-95 is 55 mph, and the original speed regulation for this roadway was 60 mph, the 85th-percentile speeds are usually around 70-72 mph. Do you know why design speeds of 65-70 mph were chosen for this report?

On the exit ramp to Route 30, the sign is actually an "advisory speed," not a "speed limit." Given how sharp the hairpin curve is, an even lower speed of 15 or 20 mph may be more appropriate than the posted "25 mph". Perhaps we can install a few supplemental high-visibility curve or chevron warning signs at some point.

On page 14, where the exit ramp from I-95 SB to I-90 is discussed, I feel the running speed on the ramp is fairly close to that of the I-95 mainline, and the horizontal curve has a fairly large radius at the exit gore, so most likely very little deceleration distance is needed.

On page 16, in the Lane Imbalance section, I would note that the entering and exiting volumes (2300-2900 vph) actually exceed the capacity of a full travel lane, making the imbalance even more severe.

That brings me to page 19 and a discussion of the exit to I-90, whether it should be a two-lane ramp vs. a one-lane ramp. We did discuss the pros and cons of each alternative, and we ended up deciding on a one-lane ramp. Some of the reasons we chose not to pursue this alternative were:

- The need for additional overhead signage (including replacement of brand-new overhead full-span sign support structures);
- Desire to discourage drivers from making the dangerous but oft-observed move from the I-95 SB center lanes to the left lane of the exit ramp by removing the second lane on the exit ramp; and
- Removal of shoulder raised safety concerns for maintenance operations using access drive from River Road (seen on Figure 13 Section 1 at the start of the 5th lane).

Costs: I'm still waiting to see the final invoices for the work we did, but my ballpark estimate for the signage (overhead and ground-mounted) is \$10-15,000 and for the pavement markings it's \$3-5,000. We still need to install final markings, reset 2 signs on permanent posts, and re-install pavement markers and rumble strips, but I think a ballpark estimate of \$25-50,000 for all the work is reasonable.

I think Figure 2 also has AM/PM swapped like Figure 9 did.

As we discussed on Wednesday, I am attaching for your information the memos that District 6 prepared for this project. The first memo was sent to FHWA on 1/22/2015 and approved by them on 2/13/2015. The second memo, requesting permanent approval for the modifications, was signed by District 6 DHD on 10/7/2015 for transmission to the Chief's office and then FHWA. Please consider the second memo a "draft" until we receive confirmation that it's been approved.

Thank you very much, I look forward to seeing the final report!

Amitai

From: Kulen, Raj (DOT)

Sent: Tuesday, October 27, 2015 12:16 **To:** Seth Asante (sasante@ctps.org)

Cc: Boudreau, Neil (DOT); Lipton, Amitai (DOT); hameed.pervez@state.ma.us **Subject:** FW: Low-Cost Improvements to Express-Highway Bottleneck Locations

Hi Seth.

This is intersecting. We have already implemented the I-93-SB lane configuration this summer as part of bridge deck work as a test and collected travel time data, now we are waiting for FHWA final approval for permanent marking.

Neil, this is good if you want to send this to FHWA as well.

Raj

From: Seth Asante [mailto:sasante@ctps.org]
Sent: Tuesday, October 27, 2015 11:29 AM

To: Kulen, Raj (DOT)

Subject: Low-Cost Improvements to Express-Highway Bottleneck Locations

Hi Raj,

The attached technical memorandum—Low-Cost Improvements to Express-Highway Bottleneck Locations is available for review.

MPO staff selected two locations for this study:

Location 1: I-93 southbound between I-95 and Montvale Avenue in Woburn and Stoneham

Location 2: I-95 southbound at the I-90 Interchange in Weston

The result of the study for Location 2, which is in MassDOT Highway Division's District 6, is presented in Section 7 of the memorandum.

Please review the attached documents and provide any comments or questions you may have by November 10, 2015.

Thank you, Seth

Seth A. Asante | Chief Transportation Planner CENTRAL TRANSPORTATION PLANNING STAFF 857.702.3644 | sasante@ctps.org www.ctps.org/bostonmpo

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Seth Asante

From: Patel, Hasmukh (DOT)

Sent: Friday, October 30, 2015 4:17 PM **To:** Seth Asante (sasante@ctps.org)

Cc: Wood, Stanley (DOT); Jasmin, Matthew (DOT)

Subject: FW: Low-Cost Improvements to Express-Highway Bottleneck Locations Study

Attachments: 2015-09-15 Low-Cost Bottlenecks MEM SA 1.pdf; Appendix A-E.pdf

Hi Seth,

I have reviewed the draft document for the subject study and have following comments.

I concur with the selection of following two locations for the study.

Location 1: I-93 Southbound between I-95 & Montvale Ave in Stoneham & Woburn

Location 3: I-95 Southbound at I-90 Interchange in Weston

Location 1: Location 1: I-93 Southbound between I-95 & Montvale Ave in Stoneham & Woburn

- Preferred Alternative 2 Create Auxiliary Lane for Merging & Diverging Traffic
 This alternative would require roadway widening. Existing right shoulder is 10 ft +-. Auxiliary lane would require
 12' wide lane and at least 6' shoulder. Anything less than 6' wide shoulder will require design exception
 approval. Also, if we go with 12' wide auxiliary lane, and 2' offset (no shoulder) to guard rail, it will require
 design exception approval. It will also require emergency pool over area.
- Cost of \$200,000-\$300,000 seems low. Consider \$500,000

Location 3: I-95 Southbound at I-90 Interchange in Weston

Concur with the improvements suggested at this interchange. No additional comments.

Hardy

From: Seth Asante [mailto:sasante@ctps.org]
Sent: Tuesday, September 15, 2015 12:41 PM

To: Patel, Hasmukh (DOT)

Subject: Low-Cost Improvements to Express-Highway Bottleneck Locations Study

Hello Hardy,

As we discussed in our recent telephone conversation, I have attached a draft document of the Boston Region MPO's Low-Cost Improvements to Express-Highway Bottleneck Locations study for review. The document has not been reviewed by the Highway Districts yet.

Please review and comment—I will address your comments before I forward it to Districts 4 and 6 for further review.

I will appreciate it if you can give me comments by Tuesday September 29, 2015.

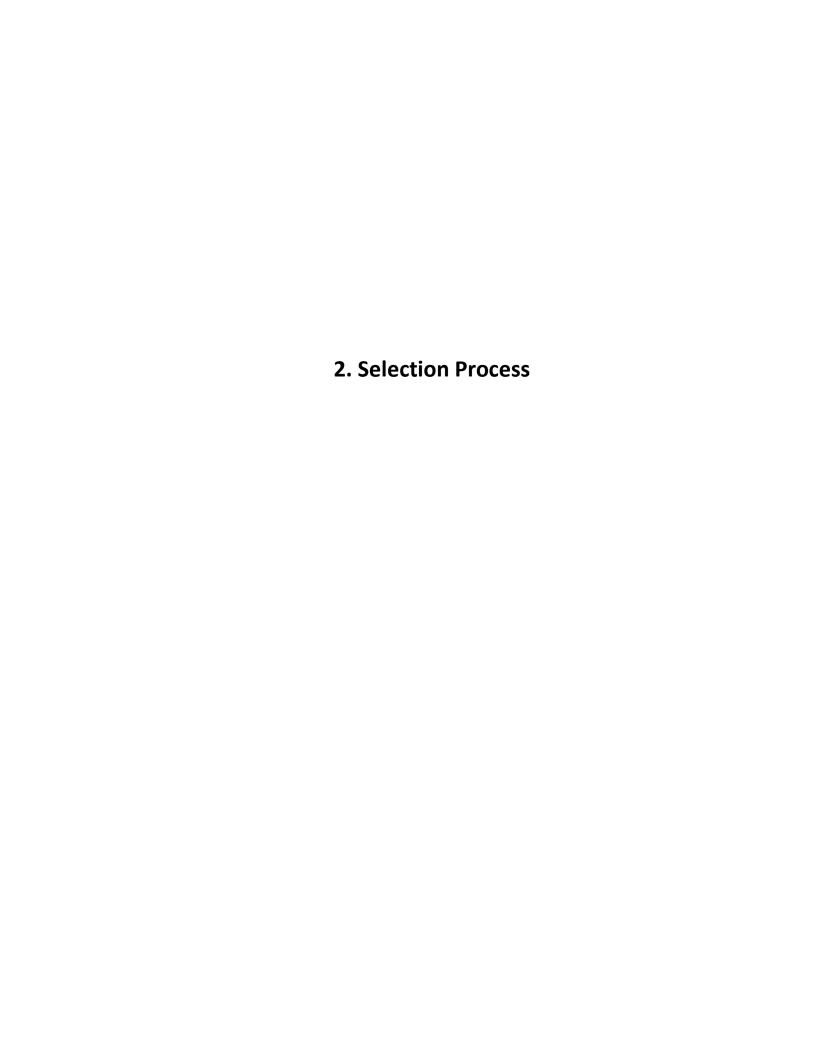
Thanks, Seth

Seth A. Asante | Chief Transportation Planner CENTRAL TRANSPORTATION PLANNING STAFF 857.702.3644 | sasante@ctps.org www.ctps.org/bostonmpo

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TECHNICAL MEMORANDUM

DATE: April 2, 2015

TO: Boston Region Metropolitan Planning Organization (MPO)

FROM: Seth Asante, MPO Staff

RE: Low-Cost Improvements to Express-Highway Bottleneck Locations

Selection of Study Locations

1 BACKGROUND

This memorandum presents the results of Task 2 of the work program for Low-Cost Improvements to Express-Highway Bottleneck Locations: FFY 2015. MPO staff indicated in Task 2—screen bottleneck locations and select locations for analysis—that we will present the results to the MPO for discussion.

According to the Federal Highway Administration (FHWA), "Much of recurring congestion is due to physical bottlenecks—potentially correctible points on the highway system where traffic flow is restricted. While many of the nation's bottlenecks can only be addressed through costly major construction projects, there is a significant opportunity for the application of operational and low-cost infrastructure solutions to bring about relief at these chokepoints."²

In the past, MPO staff analyzed several express-highway bottleneck locations in two consecutive studies, Low-Cost Improvements to Bottlenecks Phase I and Phase II, which were very well received by the Massachusetts Department of Transportation (MassDOT) and FHWA.^{3,4} Previous study locations included sections of I-95 in Weston and Burlington and sections of Route 3 in Braintree.

¹ Karl H Quackenbush, CTPS Executive Director, work program to the Boston Region Metropolitan Organization, "Low-Cost Improvements to Express-Highway Bottleneck Locations: FFY 2015," November 20, 2014.

² Federal Highway Administration, *Recurring Traffic Bottlenecks: A Primer: Focus on Low-Cost Operations Improvements*, US Department of Transportation, Federal Highway Administration, June 2009, p. 1.

³ Seth Asante, MPO staff, memorandum to the Transportation Planning and Programing Committee of the Boston Region Metropolitan Planning Organization, "Low-Cost Improvements to Bottleneck Locations, Phase I," June 2, 2011.

Chen-Yuan Wang, MPO staff, memorandum to the Transportation Planning and Programing Committee of the Boston Region Metropolitan Planning Organization, "Low-Cost Improvements to Bottleneck Locations, Phase II," dated March 12, 2012.

Some of the recommendations from those studies have been executed, such as the I-95 northbound subtract-a-lane at Interchange 24 in Weston; and FHWA has interviewed MPO staff about their successful implementation.

The cause and duration of highway bottlenecks vary. In general, recurring bottlenecks, the subject of this work program, are influenced by the design or operation present at the point where the bottleneck begins, for example: merges, diverges, lane drops, traffic weaving, abrupt changes in highway alignment, low-clearance structures, lane narrowing, intended disruption of traffic for management purposes, and less-than-optimal express-highway design. This memorandum presents the process used to select the bottleneck study locations. MPO staff will submit this proposal to the MPO for discussion and approval.

2 SELECTION OF STUDY LOCATIONS

Selection of study locations was a two-stage process that comprised inventorying and screening candidate locations.

2.1 Inventorying Candidate Locations

MPO staff developed an initial list of candidate locations in the MPO region based on the following parameters:

- Staff knowledge of bottleneck locations in the Boston MPO region
- Review of congestion management process (CMP) monitoring data and recent MPO and other planning studies
- Consultations with MassDOT Highway Division
- Input from MPO members

The inventory process yielded five bottleneck locations for screening:

- I-93 southbound between I-95 and Montvale Avenue in Stoneham and Woburn
- 2. I-93 southbound at the lane drop near Sullivan Square in Somerville/Charlestown
- 3. I-95 southbound at I-90 Interchange in Weston
- 4. Route 2 Concord Rotary
- 5. I-95 northbound, lane drop at interchange 37 in Reading, Stoneham, and Wakefield

2.2 Screening Candidate Locations

MPO staff selected two bottleneck locations for analysis. After consulting with MassDOT Highway Division, staff determined that these two locations likely could be corrected with low-cost mitigation strategies, whereas the other bottlenecks likely could not be correctible in a low-cost manner. MPO staff used the following criteria to screen the bottleneck locations:

- Does the location qualify as a bottleneck? A long traffic queue upstream trailing free-flowing traffic downstream usually characterizes the location as a bottleneck. In addition, the upstream congestion must be recurring in other words, the location experiences routine and predictable congestion because traffic volume exceeds the available capacity at that location.
- Is a physical design constraint or operational conflict inherent in the location the cause of the bottleneck? Examples of these are:
 - Lane drop—one or more travel lanes are lost, requiring traffic to merge
 - Weaving area—drivers must merge across one or more lanes in order to access an entry or exit ramp
 - Merge area—on-ramp traffic merges with mainline traffic in order to enter the freeway
 - Major interchanges—high-volume traffic is directed from one freeway to another
 - Horizontal curves—abrupt changes in highway alignment force drivers to slow down because of safety concerns
- Can the bottleneck be fixed with low-cost operational and geometric improvements? These would exclude costly long-term solutions such as expansion and major transit investments that alter driver mode choice. Examples of low-cost operational and geometric improvements are:
 - Using a short section of shoulder as an additional travel lane, an auxiliary lane, or for lengthening an acceleration or deceleration lane
 - o Restriping merge and diverge areas to better serve traffic demand
 - Providing better traveler information to allow drivers to respond to temporary changes in lane assignment, such as using a shoulder as an additional travel lane during peak periods
 - o Providing all-purpose reversible lanes
 - Changing or adding signs and striping

Based on the screening criteria and consultations with MassDOT Highway Division officials, MPO staff selected Locations 1 and 3 for study. Below are staff's rationale for not selecting Locations 2, 4, and 5:

Location 2: I-93 Southbound at the Lane Drop near Sullivan Square in Somerville/Charlestown

This section of highway is frequently congested because of a lane drop and intensive merging and diverging activities, especially during the AM peak period. During that period, the on-ramp carries between 1,300-and-1,700 vehicles per hour in an auxiliary lane; and the off-ramp to Leverett Circle, Exit 26, carries between 1,200-and-1,600 vehicles per hour. The merging and diverging activities of these vehicles slow down mainline traffic and seriously affect traffic on the upstream section on I-93. The distance between the two ramps is about 0.4 miles long. The reasons for not selecting this location are:

- Removing the lane drop would require widening the I-93 bridge over Alfred Lombardi Street to provide a new auxiliary lane for the on-ramp traffic or converting the existing auxiliary lane to an acceleration lane.
- Widening the I-93 Bridge could be expensive.
- Converting the existing auxiliary lane to an acceleration lane might create
 a queue backup on the ramp that might affect traffic on Route 38 (Mystic
 Avenue) and the collector-distributor roads. In addition, there might not be
 enough space to provide sufficient acceleration distance because of the I93 bridge over Alfred Lombardi Bridge.

Location 4: Route 2 Concord Rotary

This rotary, the intersection of Concord Turnpike (Route 2), Commonwealth Avenue, Barretts Mill Road, and Great Road (Route 119) is frequently congested because of high traffic volume and inadequate capacity during the AM and PM peak periods. The rotary is a challenge to navigate during these periods, and drivers often use local streets to avoid congestion. MassDOT Highway Division is planning to replace the rotary with an overpass for safer and more efficient operation, and to minimize environmental impacts. The Highway Division also is exploring opportunities to improve neighborhood connections, incorporate the Bruce Freeman Rail Trail and wildlife corridors, improve water and air quality, and enhance the area's design aesthetics to the extent possible.

Although this site is a major bottleneck, staff did not select this location because:

Low-cost solutions at this location likely would not be feasible.
 MassDOT and MPO staff already studied the Route 2 Concord Rotary

Page 4 of 6

⁵ Express-Highway Traffic Volumes, I-93 Southbound 2010 Balanced Traffic Volumes, Estimated by CTPS.

- to examine potential short- and long-term improvement alternatives for the rotary. ^{6,7}
- The project was removed from the funded portion of the MPO's Long Range Transportation Plan (LRTP) in August 2009 and currently is on hold.

Location 5: I-95 Northbound, Lane Drop at Interchange 37 in Reading, Stoneham, and Wakefield

This section of highway frequently is congested because of a lane drop and intensive merging and diverging activities, especially during the PM peak period, which slows down mainline traffic. During that time, the Exit 37 off-ramps carry about 3,200 vehicles per hour and the Exit 37 on-ramps carry about 2,300 vehicles per hour. Adding an auxiliary lane northbound on I-95 would provide more room for the merging and diverging activities and reduce disturbance to mainline traffic. Staff did not select this location because an auxiliary lane would need to be extended for a long distance (about three-to-four interchanges downstream) to reduce congestion and queue, which could be expensive.

3 SELECTED BOTTLENECK LOCATIONS FOR STUDY

Location 1: I-93 Southbound Between I-95 and Montvale Avenue in Stoneham and Woburn

This section of highway, about two miles long, frequently is congested because of merging and diverging activities, especially during the AM and PM peak periods. The southbound off- and on-ramps connect to and from Montvale Avenue. During peak periods, I-93 southbound carries about 8,000 vehicles per hour; the on-ramp from I-95 northbound carries about 2,000 vehicles per hour; and about 900 vehicles per hour exit to Montvale Avenue at Exit 36. In addition, about 800 vehicles per hour enter I-93 southbound from Montvale Avenue during the same period. The merging and diverging activities of these vehicles slow down I-93 southbound mainline traffic upstream of the Montvale Avenue interchange. In addition, these activities affect traffic entering I-93 southbound from I-95 northbound.

⁶ Chen-Yuan Wang, Route 2 Improvements from Route 111 in Acton to Baker Avenue in Concord: A Feasibility Study, report produced by the Central Transportation Planning Staff for the Massachusetts Department of Transportation, February 2003.

⁷ Route 2 Reconstruction at the Concord Rotary, Concord Board of Selectmen Presentation, November 24, 2008.

⁸ Express-Highway Traffic Volumes, I-95 Northbound 2007 Balanced Traffic Volumes, Estimated by CTPS.

⁹ Express-Highway Traffic Volumes, I-93 Southbound 2010 Balanced Traffic Volumes, estimated by CTPS.

Location 3: I-95 Southbound at I-90 Interchange in Weston

This bottleneck is located on I-95 southbound at the point where traffic from I-90 and Route 30 merges onto I-95. During peak periods, between 2,000-to-2,600 vehicles per hour exit I-95 southbound to I-90 and Route 30. Further downstream about the same volume of traffic enters I-95 from the same roads. However, the four I-95 southbound lanes in that section are not allocated efficiently to serve demand. As a result, during peak periods a long traffic queue forms on the I-90 and Route 30 connector ramps heading southbound on I-95.

4 SUMMARY

By identifying and evaluating a comprehensive list of potential improvements at the two locations, MPO staff will rely on their technical expertise and judgment regarding the nature of bottlenecks. MPO staff will seek input from MassDOT Highway Division staff that are familiar with the region's express-highway system operations.

This study addresses the MPO's goal of reducing congestion and increasing safety on the region's highway system. MPO staff will submit this proposal to the MPO for discussion and approval. If the MPO approves this selection, staff will meet with officials from MassDOT and discuss the study specifics, conduct field visits, collect data, and perform various analyses.

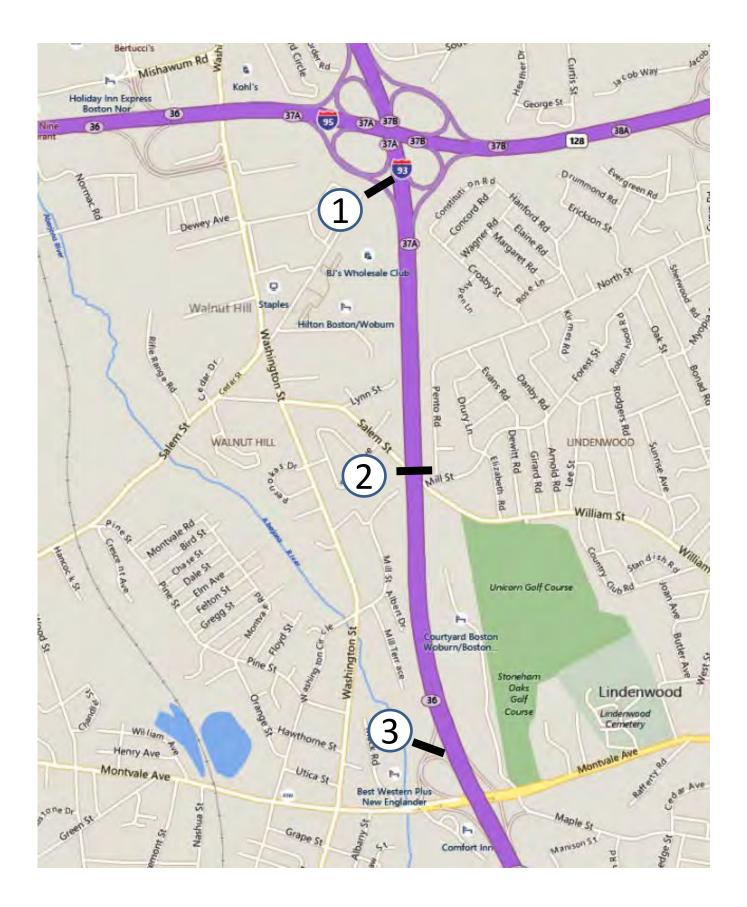
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APPENDIX B

Automatic Traffic Recorder (ATR) Count Data

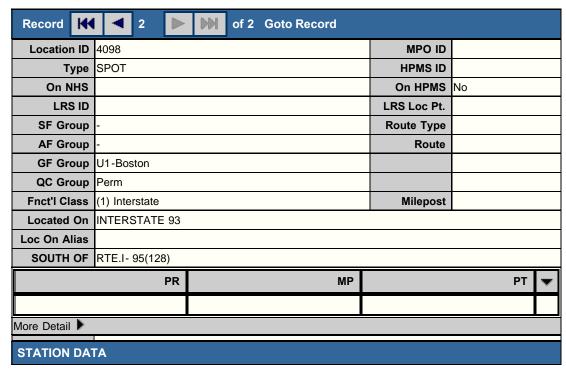
LOCATION 1

I-93 Southbound Between I-95 and Montvale Avenue in Woburn and Stoneham









Directions:



AAD1	AADT ②							
	Year	AADT	DHV-30	K %	D %	PA	ВС	Src
	2013	86,756						
	2007	92,586						

Travel Demand Model										
	Model Year	Model AADT	AM PHV	AM PPV	MD PHV	MD PPV	PM PHV	PM PPV	NT PHV	NT PPV

VOL	VOLUME COUNT					
	Date	Int	Total			
9	Tue 1/6/2015	60	91,613			
9	Mon 1/5/2015	60	92,408			
6	Sun 1/4/2015	60	63,202			
6	Sat 1/3/2015	60	77,240			
6	Fri 1/2/2015	60	90,136			
6	Thu 1/1/2015	60	56,207			
6	Wed 12/31/2014	60	93,433			
9	Tue 12/30/2014	60	97,073			
6	Mon 12/29/2014	60	94,374			
9	Sun 12/28/2014	60	70,519			
	1-10 of 339					

VOLUME TREND ②				
Year	Annual Growth			
2014	7%			
2013	-3%			
2010	0%			
2009	-2%			
2008	4%			

SPEED

CLASSIFICATION





LOCATION INFO				
Location ID	4098_SB			
Туре	SPOT			
Fnct'l Class	1			
Located On	INTERSTATE 93			
SOUTH OF	RTE.I- 95(128)			
Direction	SB			
Community	WOBURN			
MPO ID				
HPMS ID				
Agency	MHD			

COUNT DATA	INFO
Count Status	Accepted
Start Date	Mon 1/5/2015
End Date	Tue 1/6/2015
Start Time	12:00:00 AM
End Time	12:00:00 AM
Direction	SB
Notes	
Count Source	
Weather	
Study	
Speed Limit	
Description	
Sensor Type	Tube Class

INTERVAL:60-MIN				
Time	Hourly Count			
0:00-1:00	434			
1:00-2:00	275			
2:00-3:00	273			
3:00-4:00	510			
4:00-5:00	1,617			
5:00-6:00	6,466			
6:00-7:00	6,654			
7:00-8:00	5,213			
8:00-9:00	4,877			
9:00-10:00	4,994			
10:00-11:00	5,003			
11:00-12:00	4,824			
12:00-13:00	4,601			
13:00-14:00	4,674			
14:00-15:00	5,211			
15:00-16:00	5,398			
16:00-17:00	6,134			
17:00-18:00	7,619			
18:00-19:00	6,382			
19:00-20:00	3,848			
20:00-21:00	2,650			
21:00-22:00	2,104			
22:00-23:00	1,605			
23:00-24:00 📵	1,042			
Total	92,408			
AM Peak	06:00-07:00 6,654			
PM Peak	17:00-18:00 7,619			





LOCATION INFO				
Location ID	4098_SB			
Туре	SPOT			
Fnct'l Class	1			
Located On	INTERSTATE 93			
SOUTH OF	RTE.I- 95(128)			
Direction	SB			
Community	WOBURN			
MPO ID				
HPMS ID				
Agency	MHD			

COUNT DATA	INFO
COUNT DATA	INFO
Count Status	Accepted
Start Date	Tue 1/6/2015
End Date	Wed 1/7/2015
Start Time	12:00:00 AM
End Time	12:00:00 AM
Direction	SB
Notes	
Count Source	
Weather	
Study	
Speed Limit	
Description	
Sensor Type	Tube Class

INTERVAL:60-MIN				
Time	Hourly Count			
0:00-1:00	469			
1:00-2:00	323			
2:00-3:00	279			
3:00-4:00	472			
4:00-5:00	1,550			
5:00-6:00	6,518			
6:00-7:00	7,008			
7:00-8:00	5,134			
8:00-9:00	4,790			
9:00-10:00	5,220			
10:00-11:00	5,110			
11:00-12:00	4,698			
12:00-13:00	4,737			
13:00-14:00	4,557			
14:00-15:00	4,925			
15:00-16:00	5,046			
16:00-17:00	5,596			
17:00-18:00	7,085			
18:00-19:00	6,137			
19:00-20:00	4,169			
20:00-21:00	2,839			
21:00-22:00	2,177			
22:00-23:00	1,675			
23:00-24:00 📵	1,099			
Total	91,613			
AM Peak	06:00-07:00 7,008			
PM Peak	17:00-18:00 7,085			





Location ID	4098_SB	Located On	INTERSTATE 93	Community	WOBURN
Counted By	TCDS_Combined	SOUTH OF	RTE.I- 95(128)	County	MIDDLESEX
Start Date	Tue 1/6/2015	Loc On Alias		Module	
Start Time	12:00:00 AM	Direction	SB	Agency	MHD
Source	Syst_Combine	Sensor	Tube Class		
Axle Factor	0.976	Count Status	Accepted		

FHWA-S	Schem	e F Cla	assific	atio	n											
Start Time	Motor cycle	Car	Pick up	Bus	2A SU	3A SU	>3A SU	<5A 2U	5A 2U	>5A 2U	<6A >2U	6A >2U	>6A >2U	14	15	TOTAL
12:00 AM	0	430	4	5	6	3	0	4	16	1	0	0	0	0	0	469
1:00 AM	0	278	6	2	7	1	0	9	16	3	1	0	0	0	0	323
2:00 AM	0	219	7	0	7	9	0	3	27	7	0	0	0	0	0	279
3:00 AM	0	378	12	5	12	18	0	8	36	3	0	0	0	0	0	472
4:00 AM	0	1346	90	5	31	12	0	17	47	2	0	0	0	0	0	1550
5:00 AM	12	5923	404	15	39	26	0	28	67	4	0	0	0	0	0	6518
6:00 AM	14	6698	173	20	29	24	1	16	31	2	0	0	0	0	0	7008
7:00 AM	7	4708	245	28	54	23	6	13	43	7	0	0	0	0	0	5134
8:00 AM	1	4342	257	14	59	28	3	17	63	6	0	0	0	0	0	4790
9:00 AM	4	4687	262	24	80	29	6	21	99	8	0	0	0	0	0	5220
10:00 AM	8	4550	267	20	84	31	5	28	106	11	0	0	0	0	0	5110
11:00 AM	9	4180	267	15	62	45	4	33	73	9	0	1	0	0	0	4698
12:00 PM	8	4275	231	19	65	33	2	19	73	12	0	0	0	0	0	4737
1:00 PM	7	4106	247	15	50	30	3	25	68	6	0	0	0	0	0	4557
2:00 PM	9	4523	241	18	49	26	2	19	35	3	0	0	0	0	0	4925
3:00 PM	1	4710	216	16	42	19	0	13	27	2	0	0	0	0	0	5046
4:00 PM	2	5316	186	18	41	13	1	6	13	0	0	0	0	0	0	5596
5:00 PM	2	6830	174	20	31	9	0	5	12	2	0	0	0	0	0	7085
6:00 PM	1	5929	125	19	27	5	0	7	21	3	0	0	0	0	0	6137
7:00 PM	0	3988	97	9	16	7	0	11	38	3	0	0	0	0	0	4169
8:00 PM	0	2695	74	7	14	3	0	11	30	4	1	0	0	0	0	2839
9:00 PM	0	2081	53	3	11	1	0	2	19	6	0	1	0	0	0	2177
10:00 PM	1	1566	65	4	5	3	0	2	27	2	0	0	0	0	0	1675
11:00 PM	0	1027	32	4	6	2	0	5	19	3	1	0	0	0	0	1099
TOTAL	86	84785	3735	305	827	400	33	322	1006	109	3	2	0	0	0	91613





Location ID	4098_SB	Located On	INTERSTATE 93	Community	WOBURN
Counted By	TCDS_Combined	SOUTH OF	RTE.I- 95(128)	County	MIDDLESEX
Start Date	Mon 1/5/2015	Loc On Alias		Module	
Start Time	12:00:00 AM	Direction	SB	Agency	MHD
Source	Syst_Combine	Sensor	Tube Class		
Axle Factor	0.977	Count Status	Accepted		

FHWA-S	Schem	e F Cla	assific	atio	n _											
Start Time	Motor cycle	Car	Pick up	Bus	2A SU	3A SU	>3A SU	<5A 2U	5A 2U	>5A 2U	<6A >2U	6A >2U	>6A >2U	14	15	TOTAL
12:00 AM	0	375	26	4	5	0	0	5	19	0	0	0	0	0	0	434
1:00 AM	0	219	21	4	3	3	0	8	15	2	0	0	0	0	0	275
2:00 AM	1	222	20	0	5	8	0	2	13	2	0	0	0	0	0	273
3:00 AM	0	381	53	6	16	16	0	5	29	4	0	0	0	0	0	510
4:00 AM	5	1302	199	8	28	15	0	15	42	3	0	0	0	0	0	1617
5:00 AM	4	5418	869	19	44	26	0	21	61	4	0	0	0	0	0	6466
6:00 AM	8	6058	457	19	43	21	2	13	31	1	0	0	1	0	0	6654
7:00 AM	8	4619	449	28	37	19	6	12	33	0	0	2	0	0	0	5213
8:00 AM	4	4228	457	16	51	22	5	23	68	2	1	0	0	0	0	4877
9:00 AM	2	4313	446	22	65	32	7	21	79	6	1	0	0	0	0	4994
10:00 AM	5	4227	511	22	70	36	2	31	96	2	0	1	0	0	0	5003
11:00 AM	8	4059	512	16	65	34	4	22	97	7	0	0	0	0	0	4824
12:00 PM	6	3821	544	23	67	25	4	20	85	5	0	0	1	0	0	4601
1:00 PM	8	3954	477	21	71	35	6	12	83	7	0	0	0	0	0	4674
2:00 PM	4	4507	498	18	70	30	1	19	60	3	0	0	1	0	0	5211
3:00 PM	4	4818	448	18	47	19	0	10	33	1	0	0	0	0	0	5398
4:00 PM	2	5651	383	15	36	13	0	5	28	0	0	1	0	0	0	6134
5:00 PM	1	7258	280	22	28	9	0	9	9	0	0	1	2	0	0	7619
6:00 PM	7	6059	237	16	19	5	0	16	22	1	0	0	0	0	0	6382
7:00 PM	3	3671	104	11	18	6	0	13	20	2	0	0	0	0	0	3848
8:00 PM	1	2522	68	7	11	3	0	7	27	4	0	0	0	0	0	2650
9:00 PM	1	2004	48	5	11	4	0	7	21	3	0	0	0	0	0	2104
10:00 PM	1	1517	44	6	6	4	0	3	21	2	1	0	0	0	0	1605
11:00 PM	0	979	23	5	7	2	0	7	18	1	0	0	0	0	0	1042
TOTAL	83	82182	7174	331	823	387	37	306	1010	62	3	5	5	0	0	92408





Location ID	4098_SB	Located On	INTERSTATE 93	Community	WOBURN
Counted By	TCDS_Combined	SOUTH OF	RTE.I- 95(128)	County	MIDDLESEX
Start Date	Mon 12/22/2014	Loc On Alias		Module	
Start Time	12:00:00 AM	Direction	SB	Agency	MHD
Source	Syst_Combine	Sensor	Tube Class		
Axle Factor	0.975	Count Status	Accepted		

FHWA-S	Schem	e F Cla	assific	atio	n											
Start Time	Motor cycle	Car	Pick up	Bus	2A SU	3A SU	>3A SU	<5A 2U	5A 2U	>5A 2U	<6A >2U	6A >2U	>6A >2U	14	15	TOTAL
12:00 AM	0	493	22	3	4	2	0	5	16	4	0	0	0	0	0	549
1:00 AM	0	280	14	4	5	2	0	5	21	3	0	0	0	0	0	334
2:00 AM	2	241	26	0	4	11	0	5	24	6	0	0	0	0	0	319
3:00 AM	3	372	42	6	17	16	0	10	34	6	0	0	0	0	0	506
4:00 AM	2	1123	178	8	21	21	0	19	60	6	1	0	0	0	0	1439
5:00 AM	6	5190	1044	17	56	26	1	19	63	6	1	1	0	0	0	6430
6:00 AM	14	5873	564	25	51	31	0	17	27	1	0	0	0	0	0	6603
7:00 AM	4	4531	543	23	62	28	6	18	51	2	0	0	0	0	0	5268
8:00 AM	5	4358	530	17	71	25	3	22	72	4	0	1	0	0	0	5108
9:00 AM	7	4763	594	25	81	34	8	28	109	11	1	0	0	0	0	5661
10:00 AM	0	4967	629	19	86	26	4	38	105	4	0	0	0	0	0	5878
11:00 AM	1	4765	582	19	82	45	6	23	109	6	1	0	0	0	0	5639
12:00 PM	4	4598	555	12	60	43	6	14	91	6	0	0	1	0	0	5390
1:00 PM	4	4505	500	22	80	34	2	17	72	2	0	0	1	0	0	5239
2:00 PM	7	5008	504	17	61	34	4	19	50	4	3	0	0	0	0	5711
3:00 PM	1	4848	505	20	60	18	1	9	25	1	1	1	0	0	0	5490
4:00 PM	1	4850	381	16	53	6	0	4	24	1	1	0	0	0	0	5337
5:00 PM	1	5460	346	15	39	6	0	12	20	2	0	1	0	0	0	5902
6:00 PM	4	5118	258	14	39	7	0	4	29	0	1	1	0	0	0	5475
7:00 PM	1	3766	209	12	20	6	0	7	26	1	0	0	0	0	0	4048
8:00 PM	3	2880	162	6	16	5	0	13	24	1	0	0	0	0	0	3110
9:00 PM	1	2423	143	6	6	6	1	5	35	2	0	0	0	0	0	2628
10:00 PM	0	2070	123	4	9	5	0	7	25	0	0	0	0	0	0	2243
11:00 PM	0	1296	68	4	5	2	0	5	16	2	1	0	0	0	0	1399
TOTAL	71	83778	8522	314	988	439	42	325	1128	81	11	5	2	0	0	95706





Location ID	4098_SB	Located On	INTERSTATE 93	Community	WOBURN
Counted By	TCDS_Combined	SOUTH OF	RTE.I- 95(128)	County	MIDDLESEX
Start Date	Fri 12/19/2014	Loc On Alias		Module	
Start Time	12:00:00 AM	Direction	SB	Agency	MHD
Source	Syst_Combine	Sensor	Tube Class		
Axle Factor	0.98	Count Status	Accepted		

FHWA-S	Schem	e F Cla	assific	atio	n _											
Start Time	Motor cycle	Car	Pick up	Bus	2A SU	3A SU	>3A SU	<5A 2U	5A 2U	>5A 2U	<6A >2U	6A >2U	>6A >2U	14	15	TOTAL
12:00 AM	1	751	36	2	7	4	0	9	19	3	0	0	0	0	0	832
1:00 AM	1	391	22	4	2	1	0	9	14	2	0	0	0	0	0	446
2:00 AM	5	281	25	1	11	13	0	9	15	4	0	0	0	0	0	364
3:00 AM	2	422	51	2	15	15	0	15	26	4	0	0	0	0	0	552
4:00 AM	3	1179	203	14	27	14	0	39	48	4	0	0	0	0	0	1531
5:00 AM	13	5241	1019	20	38	32	1	37	54	4	0	0	0	0	0	6459
6:00 AM	10	5579	597	27	49	31	3	23	32	3	0	0	1	0	0	6355
7:00 AM	14	4608	483	24	58	25	8	17	54	4	1	2	0	0	0	5298
8:00 AM	9	4424	475	29	62	26	6	17	50	4	1	0	2	0	0	5105
9:00 AM	13	4660	533	27	80	35	4	39	79	5	0	0	1	0	0	5476
10:00 AM	6	4849	552	21	63	54	4	36	76	7	1	0	0	0	0	5669
11:00 AM	14	4973	560	21	71	37	11	42	88	4	1	0	1	0	0	5823
12:00 PM	12	4716	535	15	73	40	9	32	68	3	0	0	0	0	0	5503
1:00 PM	6	4786	530	20	51	32	5	20	38	6	0	0	0	0	0	5494
2:00 PM	11	5513	536	22	53	31	4	23	42	2	1	0	0	0	0	6238
3:00 PM	3	5741	497	22	47	14	1	12	20	1	1	2	1	0	0	6362
4:00 PM	4	6160	427	13	35	13	0	11	11	0	0	1	0	0	0	6675
5:00 PM	7	6933	301	18	27	6	0	6	12	1	0	1	0	0	0	7312
6:00 PM	4	6061	259	20	29	2	0	5	14	1	0	0	0	0	0	6395
7:00 PM	2	4361	234	13	20	10	0	9	14	0	1	0	0	0	0	4664
8:00 PM	5	3304	155	10	14	3	0	7	14	3	0	0	0	0	0	3515
9:00 PM	1	2855	139	6	12	2	0	6	15	2	0	0	1	0	0	3039
10:00 PM	0	2586	125	2	8	1	0	7	20	3	0	0	0	0	0	2752
11:00 PM	0	2046	98	9	6	4	0	3	13	3	0	0	0	0	0	2182
TOTAL	146	92420	8392	362	858	445	56	433	836	73	7	6	7	0	0	104041





LOCATION IN	LOCATION INFO					
Location ID	R12697					
Туре	SPOT					
Fnct'l Class	-					
Located On	I-95					
	Exit 37A I-93 South Boston					
Direction	RAMP					
Community	Woburn					
MPO ID						
HPMS ID						
Agency	MHD					

COUNT DATA	INFO
Count Status	Accepted
Start Date	Thu 4/10/2014
End Date	Fri 4/11/2014
Start Time	12:00:00 AM
End Time	12:00:00 AM
Direction	
Notes	
Count Source	00000000590
Weather	
Study	
Speed Limit	
Description	
Sensor Type	Tube

INTERVAL:60-MIN					
Time	Hourly Count				
0:00-1:00	199				
1:00-2:00	124				
2:00-3:00	115				
3:00-4:00	177				
4:00-5:00	363				
5:00-6:00	1,344				
6:00-7:00	1,316				
7:00-8:00	958				
8:00-9:00	1,010				
9:00-10:00	1,377				
10:00-11:00	1,604				
11:00-12:00	1,552				
12:00-13:00	1,606				
13:00-14:00	1,676				
14:00-15:00	1,823				
15:00-16:00	1,769				
16:00-17:00	1,492				
17:00-18:00	1,538				
18:00-19:00	1,669				
19:00-20:00	1,541				
20:00-21:00	1,207				
21:00-22:00	1,095				
22:00-23:00	780				
23:00-24:00 📵	405				
Total	26,740				
AM Peak	10:00-11:00 1,604				
PM Peak	14:00-15:00 1,823				





LOCATION INF	LOCATION INFO					
Location ID	R12697					
Туре	SPOT					
Fnct'l Class	-					
Located On	I-95					
	Exit 37A I-93 South Boston					
Direction	RAMP					
Community	Woburn					
MPO ID						
HPMS ID						
Agency	MHD					

COUNT DATA	INFO
Count Status	Accepted
Start Date	Tue 4/8/2014
End Date	Wed 4/9/2014
Start Time	12:00:00 AM
End Time	12:00:00 AM
Direction	
Notes	
Count Source	00000000590
Weather	
Study	
Speed Limit	
Description	
Sensor Type	Tube

INTERVAL:60-MIN	
Time	Hourly Count
0:00-1:00	178
1:00-2:00	103
2:00-3:00	92
3:00-4:00	141
4:00-5:00	376
5:00-6:00	1,322
6:00-7:00	1,228
7:00-8:00	890
8:00-9:00	767
9:00-10:00	1,127
10:00-11:00	1,497
11:00-12:00	1,447
12:00-13:00	1,520
13:00-14:00	1,604
14:00-15:00	1,768
15:00-16:00	1,877
16:00-17:00	1,837
17:00-18:00	1,781
18:00-19:00	1,882
19:00-20:00	1,402
20:00-21:00	1,084
21:00-22:00	946
22:00-23:00	623
23:00-24:00 📵	305
Total	25,797
AM Peak	10:00-11:00 1,497
PM Peak	18:00-19:00 1,882





LOCATION INFO	
Location ID	R12697
Туре	SPOT
Fnct'l Class	-
Located On	I-95
	Exit 37A I-93 South Boston
Direction	RAMP
Community	Woburn
MPO ID	
HPMS ID	
Agency	MHD

COUNT DATA	INFO
Count Status	Accepted
Start Date	Wed 4/9/2014
End Date	Thu 4/10/2014
Start Time	12:00:00 AM
End Time	12:00:00 AM
Direction	
Notes	
Count Source	00000000590
Weather	
Study	
Speed Limit	
Description	
Sensor Type	Tube

INTERVAL:60-MIN	
Time	Hourly Count
0:00-1:00	164
1:00-2:00	99
2:00-3:00	80
3:00-4:00	143
4:00-5:00	350
5:00-6:00	1,379
6:00-7:00	1,371
7:00-8:00	986
8:00-9:00	977
9:00-10:00	1,256
10:00-11:00	1,595
11:00-12:00	1,566
12:00-13:00	1,596
13:00-14:00	1,769
14:00-15:00	1,895
15:00-16:00	1,883
16:00-17:00	1,834
17:00-18:00	1,724
18:00-19:00	1,781
19:00-20:00	1,431
20:00-21:00	1,123
21:00-22:00	969
22:00-23:00	668
23:00-24:00 📵	361
Total	27,000
AM Peak	10:00-11:00 1,595
PM Peak	14:00-15:00 1,895





LOCATION INFO	
Location ID	R12697
Туре	SPOT
Fnct'l Class	-
Located On	I-95
	Exit 37A I-93 South Boston
Direction	RAMP
Community	Woburn
MPO ID	
HPMS ID	
Agency	MHD

COUNT DATA	INFO
Count Status	Accepted
Start Date	Thu 4/10/2014
End Date	Fri 4/11/2014
Start Time	12:00:00 AM
End Time	12:00:00 AM
Direction	
Notes	
Count Source	00000000590
Weather	
Study	
Speed Limit	
Description	
Sensor Type	Tube

INTERVAL:60-MIN	
Time	Hourly Count
0:00-1:00	199
1:00-2:00	124
2:00-3:00	115
3:00-4:00	177
4:00-5:00	363
5:00-6:00	1,344
6:00-7:00	1,316
7:00-8:00	958
8:00-9:00	1,010
9:00-10:00	1,377
10:00-11:00	1,604
11:00-12:00	1,552
12:00-13:00	1,606
13:00-14:00	1,676
14:00-15:00	1,823
15:00-16:00	1,769
16:00-17:00	1,492
17:00-18:00	1,538
18:00-19:00	1,669
19:00-20:00	1,541
20:00-21:00	1,207
21:00-22:00	1,095
22:00-23:00	780
23:00-24:00 📵	405
Total	26,740
AM Peak	10:00-11:00 1,604
PM Peak	14:00-15:00 1,823





LOCATION INFO	
Location ID	4098_SB
Туре	SPOT
Fnct'l Class	1
Located On	INTERSTATE 93
SOUTH OF	RTE.I- 95(128)
Direction	SB
Community	WOBURN
MPO ID	
HPMS ID	
Agency	MHD

COUNT DATA	INFO
Count Status	Accepted
Start Date	Thu 1/22/2015
End Date	Fri 1/23/2015
Start Time	12:00:00 AM
End Time	12:00:00 AM
Direction	SB
Notes	
Count Source	
Weather	
Study	
Speed Limit	
Description	_
Sensor Type	Loop

INTERVAL:60-MIN	
Time	Hourly Count
0:00-1:00	481
1:00-2:00	317
2:00-3:00	295
3:00-4:00	544
4:00-5:00	1,550
5:00-6:00	6,542
6:00-7:00	6,791
7:00-8:00	5,291
8:00-9:00	4,886
9:00-10:00	5,556
10:00-11:00	5,355
11:00-12:00	4,984
12:00-13:00	5,122
13:00-14:00	5,016
14:00-15:00	5,721
15:00-16:00	5,806
16:00-17:00	6,662
17:00-18:00	7,654
18:00-19:00	6,592
19:00-20:00	4,399
20:00-21:00	3,240
21:00-22:00	2,523
22:00-23:00	2,007
23:00-24:00 📵	1,209
Total	98,543
AADT	95,390
AM Peak	06:00-07:00 6,791
PM Peak	17:00-18:00 7,654





LOCATION INFO	
Location ID	4098_SB
Туре	SPOT
Fnct'l Class	1
Located On	INTERSTATE 93
SOUTH OF	RTE.I- 95(128)
Direction	SB
Community	WOBURN
MPO ID	
HPMS ID	
Agency	MHD

COUNT DATA INFO	
Count Status	Accepted
Start Date	Wed 1/21/2015
End Date	Thu 1/22/2015
Start Time	12:00:00 AM
End Time	12:00:00 AM
Direction	SB
Notes	
Count Source	
Weather	
Study	
Speed Limit	
Description	
Sensor Type	Loop

INTERVAL:60-MIN	
Time	Hourly Count
0:00-1:00	497
1:00-2:00	303
2:00-3:00	290
3:00-4:00	442
4:00-5:00	1,516
5:00-6:00	6,582
6:00-7:00	6,987
7:00-8:00	5,140
8:00-9:00	4,537
9:00-10:00	5,338
10:00-11:00	5,112
11:00-12:00	5,034
12:00-13:00	5,056
13:00-14:00	4,932
14:00-15:00	5,433
15:00-16:00	5,736
16:00-17:00	6,330
17:00-18:00	7,501
18:00-19:00	6,474
19:00-20:00	4,153
20:00-21:00	2,970
21:00-22:00	2,539
22:00-23:00	1,759
23:00-24:00 📵	1,074
Total	95,735
AADT	98,416
AM Peak	06:00-07:00 6,987
PM Peak	17:00-18:00 7,501





LOCATION INFO	
Location ID	4098_SB
Туре	SPOT
Fnct'l Class	1
Located On	INTERSTATE 93
SOUTH OF	RTE.I- 95(128)
Direction	SB
Community	WOBURN
MPO ID	
HPMS ID	
Agency	MHD

COUNT DATA	INFO
Count Status	Accepted
Start Date	Thu 1/15/2015
End Date	Fri 1/16/2015
Start Time	12:00:00 AM
End Time	12:00:00 AM
Direction	SB
Notes	
Count Source	
Weather	
Study	
Speed Limit	
Description	_
Sensor Type	Loop

INTERVAL:60-MIN	
Time	Hourly Count
0:00-1:00	537
1:00-2:00	330
2:00-3:00	323
3:00-4:00	491
4:00-5:00	1,570
5:00-6:00	6,132
6:00-7:00	6,840
7:00-8:00	5,009
8:00-9:00	4,134
9:00-10:00	4,799
10:00-11:00	4,849
11:00-12:00	4,881
12:00-13:00	4,986
13:00-14:00	4,975
14:00-15:00	5,495
15:00-16:00	5,622
16:00-17:00	6,146
17:00-18:00	6,465
18:00-19:00	6,650
19:00-20:00	4,276
20:00-21:00	3,046
21:00-22:00	2,588
22:00-23:00	2,031
23:00-24:00 📵	1,206
Total	93,381
AADT	90,393
AM Peak	06:00-07:00 6,840
PM Peak	18:00-19:00 6,650





LOCATION INFO	
Location ID	4098_SB
Туре	SPOT
Fnct'l Class	1
Located On	INTERSTATE 93
SOUTH OF	RTE.I- 95(128)
Direction	SB
Community	WOBURN
MPO ID	
HPMS ID	
Agency	MHD

COUNT DATA	INFO
Count Status	Accepted
Start Date	Wed 1/21/2015
End Date	Thu 1/22/2015
Start Time	12:00:00 AM
End Time	12:00:00 AM
Direction	SB
Notes	
Count Source	
Weather	
Study	
Speed Limit	
Description	
Sensor Type	Loop

INTERVAL:60-MIN	
Time	Hourly Count
0:00-1:00	497
1:00-2:00	303
2:00-3:00	290
3:00-4:00	442
4:00-5:00	1,516
5:00-6:00	6,582
6:00-7:00	6,987
7:00-8:00	5,140
8:00-9:00	4,537
9:00-10:00	5,338
10:00-11:00	5,112
11:00-12:00	5,034
12:00-13:00	5,056
13:00-14:00	4,932
14:00-15:00	5,433
15:00-16:00	5,736
16:00-17:00	6,330
17:00-18:00	7,501
18:00-19:00	6,474
19:00-20:00	4,153
20:00-21:00	2,970
21:00-22:00	2,539
22:00-23:00	1,759
23:00-24:00 📵	1,074
Total	95,735
AADT	98,416
AM Peak	06:00-07:00 6,987
PM Peak	17:00-18:00 7,501





LOCATION INFO	
Location ID	4098_SB
Туре	SPOT
Fnct'l Class	1
Located On	INTERSTATE 93
SOUTH OF	RTE.I- 95(128)
Direction	SB
Community	WOBURN
MPO ID	
HPMS ID	
Agency	MHD

COUNT DATA INFO	
Count Status	Accepted
Start Date	Tue 1/13/2015
End Date	Wed 1/14/2015
Start Time	12:00:00 AM
End Time	12:00:00 AM
Direction	SB
Notes	
Count Source	
Weather	
Study	
Speed Limit	
Description	
Sensor Type	Loop

INTERVAL:60-MIN	
Time	Hourly Count
0:00-1:00	509
1:00-2:00	322
2:00-3:00	307
3:00-4:00	493
4:00-5:00	1,598
5:00-6:00	6,608
6:00-7:00	6,443
7:00-8:00	5,269
8:00-9:00	4,748
9:00-10:00	5,457
10:00-11:00	5,419
11:00-12:00	4,934
12:00-13:00	5,022
13:00-14:00	4,763
14:00-15:00	5,226
15:00-16:00	5,648
16:00-17:00	6,440
17:00-18:00	7,298
18:00-19:00	6,550
19:00-20:00	4,236
20:00-21:00	3,158
21:00-22:00	2,382
22:00-23:00	1,809
23:00-24:00	1,068
Total	95,707
AADT	100,205
AM Peak	05:00-06:00 6,608
PM Peak	17:00-18:00 7,298





LOCATION INFO	
Location ID	R12177
Туре	SPOT
Fnct'l Class	1
Located On	I-93
	Exit 36 Montvale Ave Stoneham Woburn
Direction	RAMP
Community	Woburn
MPO ID	
HPMS ID	
Agency	MHD

COUNT DATA	INFO
Count Status	Accepted
Start Date	Wed 4/30/2014
End Date	Thu 5/1/2014
Start Time	12:00:00 AM
End Time	12:00:00 AM
Direction	
Notes	
Count Source	00000000761
Weather	
Study	
Speed Limit	
Description	
Sensor Type	Tube

INTERVAL:60-MIN	
Time	Hourly Count
0:00-1:00	65
1:00-2:00	40
2:00-3:00	23
3:00-4:00	47
4:00-5:00	112
5:00-6:00	440
6:00-7:00	545
7:00-8:00	1,373
8:00-9:00	1,237
9:00-10:00	1,077
10:00-11:00	867
11:00-12:00	737
12:00-13:00	770
13:00-14:00	791
14:00-15:00	838
15:00-16:00	777
16:00-17:00	849
17:00-18:00	1,011
18:00-19:00	747
19:00-20:00	517
20:00-21:00	416
21:00-22:00	320
22:00-23:00	250
23:00-24:00 📵	121
Total	13,970
AADT	12,208
AM Peak	07:00-08:00 1,373
PM Peak	17:00-18:00 1,011





Transportation Data Management System

Volume Count Report

LOCATION IN	- 0
Location ID	R12177
Туре	SPOT
Fnct'l Class	1
Located On	I-93
	Exit 36 Montvale Ave Stoneham Woburn
Direction	RAMP
Community	Woburn
MPO ID	
HPMS ID	
Agency	MHD

COUNT DATA	INFO
Count Status	Accepted
Start Date	Tue 4/29/2014
End Date	Wed 4/30/2014
Start Time	12:00:00 AM
End Time	12:00:00 AM
Direction	
Notes	
Count Source	00000000761
Weather	
Study	
Speed Limit	
Description	
Sensor Type	Tube

INTERVAL:60-N	IIN
Time	Hourly Count
0:00-1:00	54
1:00-2:00	31
2:00-3:00	38
3:00-4:00	48
4:00-5:00	106
5:00-6:00	456
6:00-7:00	596
7:00-8:00	1,368
8:00-9:00	1,006
9:00-10:00	912
10:00-11:00	814
11:00-12:00	735
12:00-13:00	779
13:00-14:00	795
14:00-15:00	832
15:00-16:00	837
16:00-17:00	871
17:00-18:00	1,036
18:00-19:00	727
19:00-20:00	506
20:00-21:00	356
21:00-22:00	276
22:00-23:00	209
23:00-24:00 📵	115
Total	13,503
AADT	11,982
AM Peak	07:00-08:00 1,368
PM Peak	17:00-18:00 1,036





Transportation Data Management System

Volume Count Report

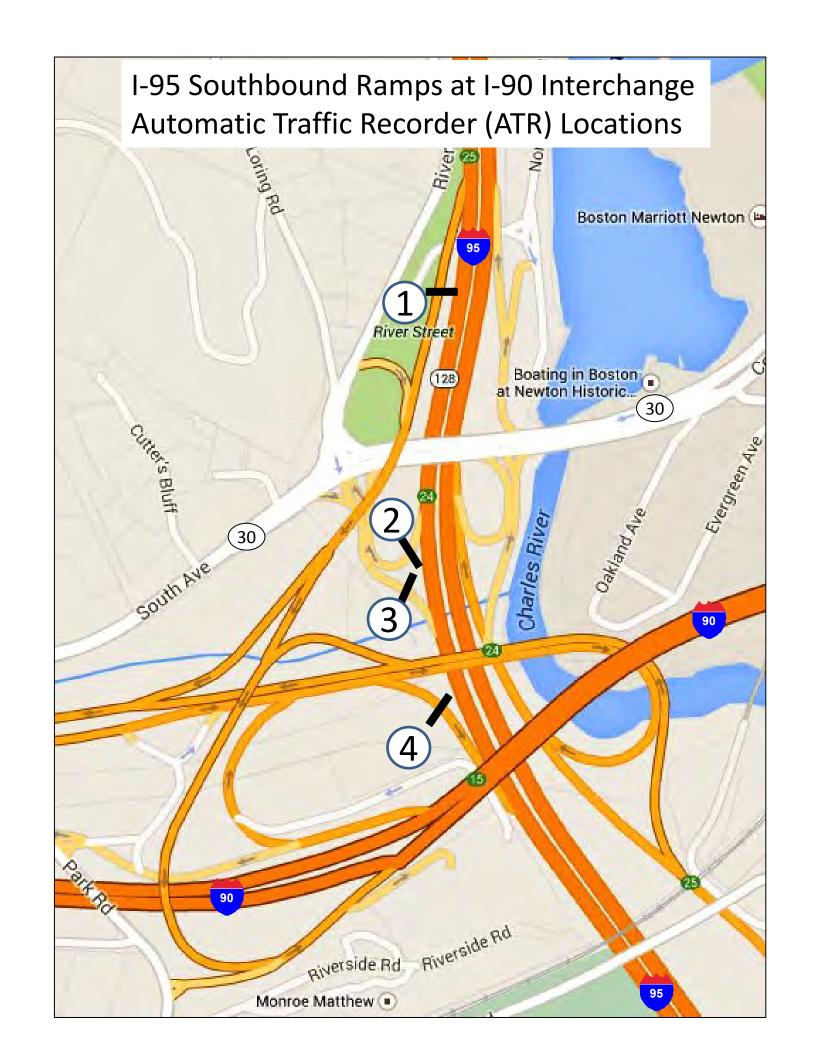
LOCATION INI	FO
Location ID	R12177
Туре	SPOT
Fnct'l Class	1
Located On	1-93
	Exit 36 Montvale Ave Stoneham Woburn
Direction	RAMP
Community	Woburn
MPO ID	
HPMS ID	
Agency	MHD

COUNT DATA	INFO
Count Status	Accepted
Start Date	Mon 4/28/2014
End Date	Tue 4/29/2014
Start Time	12:00:00 AM
End Time	12:00:00 AM
Direction	
Notes	
Count Source	00000000761
Weather	
Study	
Speed Limit	
Description	
Sensor Type	Tube

INTERVAL:60-N	IIN
Time	Hourly Count
0:00-1:00	48
1:00-2:00	30
2:00-3:00	37
3:00-4:00	46
4:00-5:00	117
5:00-6:00	412
6:00-7:00	529
7:00-8:00	1,132
8:00-9:00	997
9:00-10:00	884
10:00-11:00	712
11:00-12:00	705
12:00-13:00	691
13:00-14:00	698
14:00-15:00	804
15:00-16:00	844
16:00-17:00	769
17:00-18:00	958
18:00-19:00	670
19:00-20:00	483
20:00-21:00	275
21:00-22:00	270
22:00-23:00	192
23:00-24:00 📵	90
Total	12,393
AADT	11,345
AM Peak	07:00-08:00 1,132
PM Peak	17:00-18:00 958

LOCATION 2

I-95 Southbound at the I-90 Interchange in Weston



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STA.1

Site Reference: 150110000624 Site ID: 000000000100 Location: EXIT 25 RAMP FROM I-95 SB TO I-90 Direction:

File: V100.prn City: WESTON County: VOL

TIME	MON	TUE 28	WED 29	THU	FRI 1		SAT 2	SUN 3	WEEK	TOTAL
01:00	184	219	192	190		206				1575
02:00	118	165	168	120	198	153	176	320	180	1265
03:00	118 84 112	117	128	137	132	119 154	160	135	127	893 985
04:00	112	173	150	149	189	154	123	89	140	985
05:00	291	282	284	302	321	296	213	105	256	1798
06:00	822	877	874	815		842	376	188	682	4778
07:00	1880	2056	1743		2 22 A 22	1857	637	466	1484	10392
08:00	1950	1631	1634	1604	1834	1730	1013	604	1467	10270
09:00	1378	1352	1568	1.650	1834 1970	1583	1368	822	1467 1444	10108
10:00	1622	1654	1713	1803	1868	1732	1591	1285	1648	11536
11:00	1800	1636	1734			1738			1747	
12:00	1623	1644	1605	1724	1958	1710	1835	1767	1736	12156
13:00	1500	1718	1637	1755	2032	1730	1896			
14:00	1508	1624	1604	1874	1978	1717	1741	1746	1753 1725	12075
15:00	1500 1508 1881	1973	2134	2241	2190		1709	1819	1992	13947
16:00	2174	2305	2409	2220		2239	1695		2089	
	2029			2268		2220			2062	
18:00	1885 2101 1262	2140	2405	2135	2032				1966	
19:00	2101	2157	2037	2096	1637	2005	1613	1443	1869	13084
20:00	1262	1431	1359	2135 2096 1401	1495	1389	1167	1039	1307	9154
21:00	937	958	1122	1058	995	1014	960	886	988	6916
22:00		837	890	859	802	847	887	618	815	4893
23:00		580	617	560	678	847 608	607	389	571	3431
24:00		350	345	375	495	608 391	440	238	373	2243
TOTALS	27141	30132	30814	30949	31584	30482	25608	22599	28646	198827
AVG WKDY	00	00 0	7.07	101.5	102 6		9.4	74.1		
AVG WEEK	94.7	105.1		101.5	110.2			78.8		
AM Times	08:00	07:00	07:00	07:00	09:00	07:00	11:00	12:00	11:00	
	1950					1857				
PM Times						16:00			16:00	
PM Peaks	2174	2305	2462	2268	2190	2239	1896	1819	2089	

Page; 1

STA. 2

Hite Reference: 150110000862 Hite ID: 000000000200 Hocation: EXIT 24 RAMP FROM I-95 SB TO RTE 30 Hirection:

File: V200.prn City: WESTON County: VOL

TIME	MON 4	TUE 28	WED 29	THU 30	FRI 1	WKDAY	SAT 2	SUN 3	WEEK	TOTAL
01:00	16	25	33	15	26	23	30	47	27	192
02:00	12	19	17	12	10	14	29	53	21	152
03:00	9 8 24	13	9	12	7	10	18	28	13 8 21	96
04:00	8	6	17	9	6	9	5	11	8	62
05:00	24	30	31	28	18	26	11	В	21	150
06:00	114	143	137	122	109	125	36	27	98	688
07:00	509			490	521	546	134	61	418	
08:00					778	875	215	100	670	4691
09:00	766 590	755	788	690	652	875 695	242	135	550	3852
10:00	591	690	636	642	462	604	293	100 135 209	503	3523
11:00	397				429	454	369	298	419	2937
12:00	357	440	427	388	356	393	363	245	202	
13:00	353	431	459	527	448	443	388	387	427	
14:00	351	431 435	430	451	456	424	420	387 389	418	
15:00	459	472	458	468	554	482	395	420	460	
	554									
17:00		592			515	545	384	417	504	
18:00	605	736	887	732	746	741	1115	300	650	
19:00	615	736 684	670	595	493	741 611	369	320	650 535	3746
20:00	325	347	378	339	281	334	274	251	313	2195
	233	250	255	223	192	230	192	141	212	1486
22:00	400	166	178	141	155	160	201	149	1.65	990
23:00		95	106	100	132	108	148	83	110	554
24:00	200	48	52	53	72	56	76	35	56	336
TOTALS		9248								52046
& AVG WKDY	87.5	109.7	107-8	98.2	92.9		65.3	55.9		
	98.7			110.7			73.7	63		
AM Times						08:00			08:00	
AM Peaks	766	1115	901	816	778	875	369	345	670	
PM Times						18:00				
PM Peaks	615	736	887	732	746	741	470	420	650	

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STA.3

Site Reference: 150110000467 Site ID: 000000000300 Location: ON-RAMP FROM RTE. 30 TO I-95 SB Direction:

File: V300.prn City: WESTON County: VOL

TIME	MON 4	TUE 28	WED 29	THU 30	FRI 1	WKDAY AVG	SAT 2	SUN 3	WEEK	TOTAL
01:00	31	30	40	33	40	34	56	62	41	292
					35	24	30	81		231
03:00	15	17 15 26	19	27	22	19	20	26	20	144
04:00	21	26	21	13	27	21	13	16	19	137
05:00	33	26 24	37	32	27	30	22	1.4	27	189
06:00	93	112	105		99	103	46	34	85	596
07:00	329	330	337	329	335	332	120	89	267	1869
08:00	506	544	541	557	543	538	252	7.70	444	3113
08:00	557	561	546	544	566	538 554	341	219	476	3334
10:00	429	444	480	477	458	457	393	296	425	2977
	412		421			411				2823
12:00	389	385	400	408	418	400	419	388	401	2807
13:00		440	414	456	444	432	432	439	433	3034
14:00	397	435	453	462	462	432 441	379	389	425	2977
15:00	466	461	470	505	505	481	407	376	455	3190
		494	467	455		468		368	442	3096
17:00	422	539	557	543				309	458	3210
18:00	518	524	515	490	497	508	372	291	458	3207
19:00	436	524 427	452	466	396	435	283	214	382	2674
20:00	300	351	300			316		218	297	2079
		271		228	210	232	187	152	214	1502
22:00	4,4,4	253	206	178	154	197	224	153	194	1168
23:00		127	98	110	194	132	195	81	134	905
24:00		253 127 82	80	76	137	93	112	52	194 134 89	539
TOTALS	6448	7276	7235			7169				45993
& AVG WKDY	89.9	101.4	100.9	101.5	100.9		80.2	66.4		
& AVG WEEK	97.3	109.8	109.2							
AM Times			09:00	08:00		09:00		12:00		
AM Peaks	557	561	546	557	566	554	439	388	476	
	18:00					17:00				
PM Peaks	518	539	557	543	505	511	432	439	458	

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STA. 4

Site Reference: 150110000625 Site ID: 000000000400 Location: ON-RAMP FROM I-90 TO I-95 SB Direction: File: V400.prn City: WESTON County: VOL

TIME	MON 4	TUE	WED	THU 30	FRI	WKDAY	SAT	SUN	WEEK	TOTAL
	4	4.0	29	30		AVG		3	AVG	
01:00	171	164	208	215	303	212	276	263	228	1600
	135	108	106	139	159	129	190	276	159	1113
03:00	77	90 80 192	63	76	107	82 91 172	127	151	98	691
04:00	91	90	63 86	97	104	91	87	65	87	610
05:00	148	192	173	177	172	172	79	45	140	610 986
06:00	568	552	600	562	575	571	175	107	448	3139
07:00	1080	1198	1248	1265	1276	1213	432	280	968	6779
08:00	1736	1896	1861	1874	1795	1832	672	440	1467	10274
09:00	1736 1774 1719	1849	1760	1771	1833	1832 1797 1749	987	572	1506	10546
10:00	1719	1788	1815	1851	1575	1749	1177	805	1532	10730
11:00	1273	1368	1186	1430	1448	1341	1428	1181	1330	9314
	1177	1220		1333	1347	1254			1327	
13:00	1244	2007	1 222		4.4	1293			1351	
14:00	1244 1255 1494	1247 1563	1475	7263	1512			1418	1376	9632
15:00	1494	1563	1588	1570	1571	1557	1564	1464	1376 1544	10814
16:00	1370	1566	1.450	1446	1428	1452	1441	1526	1.461	10227
m mm . m m						1321			1348	
18:00	1185 1345 1253 968 675	1363	1410	1346	1366	1366	1337		1367	
19:00	1253	1342	1323	1419	1491	1365	1080	1140	1292	9048
20:00	968	1128	1052	1047	1198	1365 1078	917	986	1042	7296
21:00	675	731	748	748	763	733	700	768	733	5133
22:00		558	727	704	718	676	713	617	672	4037
23:00		429	526	521	636	528	726	441	546	3279
24:00		307	298	368	455	357	473	306	367	2207
rotals	20738			23837		23519			22389	155222
AVG WKDY	88.1	99.4	100.3	101.3	104.7		88 4	78		
AVG WEEK	92.6	104.5	105.4		109.4		92.9	81.9		
AM Times						08:00				
AM Peaks	1774	1896	1861	1874	1833	1832	1688	1333	1532	
PM Times						15:00		16:00		
PM Peaks	1494	1566	1588	1570	1571	1557	1638	1526	1544	

STA.1

Site Reference: 150110000624 Site ID: 000000000100 Location: EXIT 25 RAMP FROM I-95 SB TO I-90 Direction: ROAD TOTAL

File: V100.prn City: WESTON County: VOL

	TIME	M	ON 4		UE 28		ED 29	T	HU 30	E	RI 1	S	AT 2	S	UN 3	WK	TOT	WK	AVG
Lane	2	am	pm	am	рш	am	pm	am	pm	am	pm	am	pm	am	pm	am	pm	am	pm
(00:15	53	400	64	445	64	390	45	453	75	497	89	438	75	390	465	3013	66	430
	00:30	58	372	47	429	43	470	39	488	70	527	69	448	64	434			55	452
	00:45	33	354	54	430	45	382	64	393	63	526	70	538	74	429		3052	57	436
	01:00	40	374	54	414	40	395	42	432	37	482 .	49	472	55	470		3039	45	434
	01:15	27	351	46	389	67	407	40	456	56	468	64	481	87	453		3005	55	429
	01:30	28	439	51	410	33	374	22	458	57	475	41	426	111	474		3056	49	436
	01:45	35	352	39	397	39	403	27	451	42	498	32	407	61	409		2917	39	416
	02:00	28	366	29	428	29	420	31	509	43	537	39	427	61	410		3097	37	442
	2:15	12	424	31	435	45	490	27	546	31	532	40	408	52	478		3313	34	473
	02:30	16	410	15	505	13	500	26	541	43	596	50	417	30	482		3451	27	493
	02:45	23	503	30	512	44	541	40	563	29	544	43	463	26	445		3571	33	510
	03:00	33	544	41	521	26	603	44	591	29	518	27	421	27	414		3612	32	516
	03:15	22	557	32	561	31	612	22	583	54	513	30	430	17	413		3669	29	524
	03:30	31	544	46	569	28	638	32	540	47	505	30	408	24	449		3653	34	521
	03:45	32	488	48	585	42	609	28	565	42	535	25	446	28	431		3659	35	522
	04:00	27	585	47	590	49	550	67	532	46	537	38	411	20	439		3644	42	520
	04:15	52	589	49	603	48	655	57	515	61	548	44	433	18	434		3777	47	539
	04:30	64	557	69	522	67	579	63	559	58	531	45	410	25	433		3591	55	513
	34:45	69	471	75	574	70	581	80	581	77	463	73	372	39	481		3523	69	503
	05:00	106	412	89	554	99	647	102	613	125	547	51	368	23	402		3543	85	506
	05:15	114	487	122	583	129	661	99	561	101	502	75	361	35	407		3562	96	508
	05:30		461									89							
		174		166	452	162	557	159	531	133	497		424	47	364		3286	132	469
	05:45	236	450	247	556	258	570	243	491	262	587	87	463	54	365	1387		198	497
	06:00	298	487	342	549	325	617	314	552	330	446	125	381	52	405	1786		255	491
	06:15	449	538	482	567	419	578	444	483	394	430	107	425	74	352	2369		338	481
	06:30	479	554	527	550	519	496	528	588	424	398	145	401	92	418		3405	387	486
	06:45	473	556	544	541	406	500	447	591	448	397	197	451	170	358	2685		383	484
	07:00	479	453	503	499	399	463	450	434	475	412	188	336	130	315	2624		374	416
	07:15	505	382	480	449	386	398	404	438	460	416	195	305	105	285		2673	362	381
	07:30	608	314	399	372	433	367	437	391	451	391	254	328	137	276		2439	388	348
	07:45	458	307	400	313	373	331	382	303	503	343	295	283	178	250		2130	369	304
	08:00	379	259	352	297	442	263	381	269	420	345	269	251	184	228		1912	346	273
	08:15	370	239	370	274	413	284	432	274	500	276	292	267	187	216		1830	366	261
	08:30	351	229	306	256	372	258	409	296	457	253	361	229	188	250		1771	349	253
	08:45	316	214	339	214	397	326	400	251	497	251	392	233	211	210		1699	364	242
	09:00	341	255	337	214	386	254	409	237	516	215	323	231	236	210		1616	364	230
	09:15	305		367	215	378	258	435	222	432	191	346	239	270	159		1284	361	214
(09:30	382		396	236	398	208	449	241	493	210	411	234	326	165		1294	407	215
1	09:45	432		422	215	458	231	462	182	433	209	426	204	345	142		1183	425	197
1	10:00	503		469	171	479	193	457	214	510	192	408	210	344	152	3170	1132	452	188
	10:15	529		453	187	436	171	461	147	479	171	470	172	370	124	3198	972	456	
	10:30	432		371	155	439	162	419	153	429	198	491	164	366	95	2947	927	421	154
- 3	10:45	446		396	131	431	140	432	151	436	165	483	151	426	93	3050	831	435	138
	11:00	393		416	107	428	144	421	109	445	144	468	120	464	77	3035	701	433	116
	11:15	425		386	95	372		406	118	514	136	442	127	423	58	2968	615	424	102
	11:30			419	97		106	464	103	486	143	470	125	452	63	3111	637	444	106
	11:45			407	84	417	93	421	94	482	113	441	93	399	61	2961	538	423	89
	12:00				74		65		60		103		95		56				75
TOT	ALS	27	141	30	132	30	0814	30	949	31	584	21	5608	22	599	19	8827	2	8617
AM	Peaks		2065		2056		1812		1869		1970		1912		1767		10:00 12365		176
PM '	Times		15:45 2219		15:30		16:30		16:30								15:30 14733		15:3 210

Page: 1

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STA.2

Site Reference: 150110000862

Site ID: 000000000200 Location: EXIT 24 RAMP FROM I-95 SB TO RTE 30-Direction: ROAD TOTAL

File: V200.prn City: WESTON County: VOL

	TIME	M	ION 4		UE 28		ED 29		HU 30	F	RI 1	S	AT 2	S	UN 3	WK	TOT	WK	AVG
Lane	2	am	pm	am	pm	am	pm	am	pm	am	pm	am	pm	am	pm	am	pm	am	pm
								-140-											
1	00:15	2	91	8	100	3	132	6	145	11	101	10	82	16	92	56	743	8	106
	30:30	2	84	11	114	6	113	5	151	7	95	9	110	14	76	54	743	7	106
	00:45	6	100	4	119	15	116	2	100	3	136	7	91	6	117	43	779	6	111
	01:00	6	78	2	98	9	98	2	131	5	116		105	11	102	39	728	5	104
	01:15	4	83	12	98	2	107	1	113	2	106	9	108	10	101	40	716	5	102
	01:30	3	82	2	129	2	94	2	99	4	113	11	102	24	102	48	721	5	103
	01:45	5	73	1	106	6	122	5	108	2	117	4	103	14	105	37	734	5	104
	02:00	0	113	4	102	7	107	4	131	2	120	5	107	5	81	27	761	3	108
	02:15	1	97	2	103	1	98	7	B7	2	117	4	109	11	113	28	724	4	103
	02:30	2	107	1	126	3	111	3	103	3	117	3	103	5	90	20	757	2	108
	02:45	5	122	3	118	3	119	1	129	1	165	8	92	6	126	27	871	3	124
	03:00	1	133	7	125	2	130	1	149	1	155	3	91	6	91	21	874	3	124
	03:15	2	143	1	134	2	126	2	144	1	114	1	114	3	106	12	881	1	125
	03:30	2	151	1	145	12	142	1	126	1	98	3	124	2	92	22	878	3	125
	03:45	3	142	3	183	3	139	2	119	2	108	0	124	2	110	15	925	2	132
13	04:00	1	118	1	131	0	139	4	84	2	95	1	108	4	93	13	768	1	109
	04:15	2	116	3	121	1	151	6	112	4	96	2	89	3	101	21	786	3	112
10	04:30	5	118	5	149	9	149	6	113	1	138	5	96	0	110	31	873	4	124
	04:45	8	115	9	135	6	163	4	102	3	146	0	103	1	90	31	854	4	122
19	05:00	9	142	13	187	15	188	12	152	10	135	4	96	4	116	67	1016	9	145
13	05:15	11	118	9	165	6	191	14	160	12	171	8	106	1	106		1017	8	145
.0	05:30	21	160	25	170	27	229	14	178	24	184	8	109	5	94		1124	17	160
1	05:45	26	157	53	166	41	212	38	208	26	209	9	114	8	79		1145	28	163
0	06:00	56	170	56	235	63	255	56	186	47	182	11	117	13	120	302	1265	43	180
11	06:15	66	147	83	221	70	239	74	175	77	172	11	95	11	95	392	1144	56	163
1	06:30	115	158	146	188	122	172	93	142	107	118	30	92	16	86	629	956	89	136
Q)	06:45	140	171	182	156	161	142	134	155	146	102	38	96	12	86	B13	908	116	129
	07:00	188	139	260	119	188	117	189	123	191	101	55	86	22	53	1093	738	156	105
- N	07:15	193	90	281	127	240	115	198	110	211	71	47	66	24	58	1194	637	170	91
_0	07:30	219	83	327	82	262	103	246	94	207	86	46	91	22	7.4	1329	613	189	87
10	07:45	177	90	268	74	196	82	185	75	174	64	54	52	30	53	1084	490	154	70
	00:80	177	62	239	64	203	78	187	60	186	60	68	65	24	66	1084	455	154	65
	08:15	158	56	204	96	204	72	199	58	190	47	52	52	22	37	1029	418	147	59
11	08:30	165	69	216	54	178	69	167	51	170	49	44	52	37	40	977	384	139	54
	08:45	139	62	173	48	218	62	165	52	138	47	65	47	37	29	935	347	133	49
	09:00	128	46	162	52	188	52	159	62	154	49	81	41	39	35	911	337	130	48
	09:15	137		202	51	175	48	171	34	126	41	5.5	61	48	40	914	275	130	45
	09:30	141		155	34	151	57	196	35	111	33	77	51	50	47	881	257	125	42
	09:45	181		148	38	146	42	135	40	105	36		45	48	35	848	236	121	39
	10:00	132		185	43	164	31	140	32	120	45	76	44	63	27	880	222	125	37
	10:15	130		139	33	125	33	134	30	132	35	90	32	71	27	821	190	117	31
	10:30	91		109	22	119	28	106	29	101	36	103	34	63	22	692	171	98	28
	10:45	95		122	26	129	21	128	24	91	39	90	40	70	21	725		103	28
	11:00	81		122	14	108	24	103		105	22	86	42	94	13		132	99	22
	11:15				17			95			26				9			90	
	11:30						7		14		15				14	652			13
	11:45				10			73		81	21				4			96	
									12	96					8				9
	ALS		7379										508		1714		2046		7441
AM	Times		7:00		7:00		7:15		7:00		7:00		10:30 370		11:00		7:00		7:00
			18:00										15:15						
	Peaks		646		B10				747				470		422		4678		666

STA.3

Site Reference: 150110000467 Site ID: 000000000300

Location: ON-RAMP FROM RTE, 30 TO I-95 SB Direction: ROAD TOTAL

File: V300.prn City: WESTON County: VOL

	TIME	M	ON 4		UE 28		ED 29		НU 30	F	RI 1	5	AT 2	S	UN 3	WK	TOT	WK	AVG
Lane	2	am	pm	am	pm		pm	am	pm	am	pm	am	pm	am	pm	am	pm	am	pm
0	0:15	9	93	11	91	18	112	10	111	14	100	21	102	17	101	100	710	14	101
	0:30	5	96	4	131	4	104	9	114	В	109	9	117	16	125	55	796	7	113
	0:45	10	104	11	97	10	101	7	128	12	121	15	96	12	112	77	759	11	108
	1:00	7	116	4	121	8	97	7	103	6	114		117	17	101	60	769	8	109
	1:15	4	93	4	115	15	106	11	109	12	121	14	88	32	103	92	735	13	105
0	1:30	6	100	3	105	6	118	3	113	11	122	3	103	27	90	59	751	8	107
	1:45	1	102	5	117	5	122	4	126	4	120	7	96	17	108	43	791	6	113
	2:00	2	102	5	98	7	107	4	114	8	99	6	92	5	88	37	700	5	100
	2:15		104	2	105	4	106	11	104	4	133	7	104	8	110	40	766	5	109
Ő	2:30	5	128	6	117	2	140	5	140	6	146	6	102	6	102	36	875	5	125
	2:45	3	117	1	126	5	126	6	140	6	117	6	99	7	79	34	804	4	114
	3:00	3		6	113	8	98	5	121	6	109	1	102	5	85	34	745	4	106
	3:15	4	134	6	118	6	126	4	119	4	123	2	97	4	91	30	8.08	4	115
	3:30	9		6	140	В	139	4	112	5	108	2	103	5	89	39	797	5	113
	3:45	4		8	123	1	101	3	100	8	109	5	97	3	87	32	724	4	103
	4:00	4		6	113	6	101	2	124	10	124		90	4	101	36	767	5	109
	4:15	2	132	0	139	9	131	2	127	2	104	6	76	2	74	23	783	3	111
	4:30		111	8	133	7	123	2	140	6	131	10	91	3	94	39	823	5	117
	4:45	15	104	4	136	10	142	14	133	7	135	2	80	5	74	57	804	8	114
	5:00	13	75	12	131	11	161	14	143	12	126	4	97	4	67	70	800	10	114
			116	16		13	140	19	145	15	131	9	81	10	79	100	851	14	121
0	5:15	20		15	159	14	141	14	136	14	120	9	100	5	65	91	832	13	118
			125		145					36	119	13	95	6	78	205		29	110
	15:45		137	45 36	123	37	112	41	106			15	96	13	69	200	770 754	28	
			140		97	41	122	33	103	34	127								107
	6:15		134	51 59	108	61 65	111	74	112 125	54 65	116 97	22	74 64	18	50 66	313	705 692	52	100
	6:30		101			95						35	71	26	45	601	693	85	99
	6:45	98	106	117	127		113	112	125	118	106 77	38	74	24		588	584		
	7:00	110	95	103	83	116	98	99	104						53			84	83
	7:15	118	93	130	102	137	85	127	86	135	88	49	81	44	66	740	601	105	85
	7:30	122	76	128	93	121	77	127	92	121	77	68	79	43	51	730	545	104	77
	7:45	135	67	140	73	142	80	146	76	149	64	72	60	41	55	825	475	117	67
	00:80	131	64	146	83	141	58	157	95	138	54	63	58	42	46	818	458	116	65
	8:15	143	56	153	88	146	91	132	76	139	57	76	47	63	43	852	458	121	65
	8:30	152	58	120	62	127	55	149	60	152	54	84	62	48	37	832	388	118	55
	8:45	135	46	145	61	151	50	133	51	142	55	91	44	60	38	857	345	122	49
	9:00	127	51	143	60	122	47	130	41	133	44	90	34	48	34	793	311	113	44
	9:15	113		130	84	123	48	124	59	140	44	80	61	66	37	776	333	110	55
	9:30	106		114	82	124	64	128	51	120	40	100	52	74	57	766	346	109	57
	9:45	112		106	56	130	53	117	35	109	28	108	63	77	29	759	264	108	44
	0:00	98		94	31	103	41	108	33	89	42	105	48	79	30	676	225	96	37
	0:15	105		92	35	107	31	110	23	97	38	104	39	72	21	687	187	98	31
	0:30	93		93	33	117	27	92	30	107	69	103	49	101	21	706	229	100	38
	10:45	101		102	27	105	22	104	22	108	44	116	60	86	23	722	198	103	33
	1:00	113		97	32	92	18	108	35	114	43	116	47	68	16	708	191	101	31
	11:15						31	96	25				43	86	24	666	197		
	1:30					81		92			35				14			95	
	1:45					108		123			33				11		135	107	
1	12:00	97		95	12	115	15	97	15	109	30	99	20	109	3	721	95	103	15
TOTA	ALS		5448		1276		7235	-	7281		7238		5751		1764	45	993	7777	6586
AM 7	Cimes		7:45		7:30		8:00		7:45		7:45		10:45		11:15		8:00		8:00
													10:45 446						
PM 7	Times		17:30		16:45		16:45		16:30		16:30		12:15		12:30		16:45		16:45
PM I	Peaks		536		571		584		561		523		432		441		3287		467

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Site Reference: 150110000625 Site ID: 000000000400 Location: ON-RAMP FROM I-90 TO I-95 SB Direction: ROAD TOTAL

STA.4 File: V400.prn City: WESTON County: VOL

	TIME	М	ON 4		UE 28		ED 29		HU 30	F	RI 1	S	AT 2	S	UN 3	WK	TOT	WK	AVG
Lane	2	am	pm	am	pm	am	pm	am	pm	am	pm	am	pm	am	pm	am	pm	am	pm
2-2-					-4														
C	00:15	53	337	47	246	43	321	69	317	80	293	83	427	70	346	445	2287	63	326
	00:30	46	279	32	316	54	310	45	313	70	317	73	416	70	287		2238	55	319
	0:45	35	302	40	368	45	352	50	310	76	358	56	383	64	358		2431	52	347
	1:00	37	326	45	341	66	348	51	342	77	373	64	412	59	365	399	2507	57	358
	1:15	40	310	27	286	36	341	35	274	48	354	63	365	64	349		2279	44	325
C	01:30	37	311	28	323	26	346	44	310	44	353	46	352	84	371		2366	44	338
C	1:45	30	326	21	300	26	424	33	300	32	403	36	386	62	367	240	2506	34	358
C	2:00	28	308	32	338	18	364	27	379	35	402	45	359	66	331		2481	35	354
C	2:15	20	342	31	370	21	353	24	355	37	411	41	386	64	340	238	2557	34	365
. (2:30	20	381	25	410	20	422	17	377	26	407	28	386	29	385	165	2768	23	395
C	12:45	19	382	16	384	15	427	25	478	28	374	34	419	31	361	168	2825	24	403
C	00:80	18	389	18	399	7	386	10	360	16	379	24	373	27	378	120	2664	17	380
C	03:15	18	368	11	388	11	374	24	364	28	382	16	347	16	342	124	2565	17	366
	3:30	23	290	27	419	17	376	23	383	18	322	25	349	24	393		2532	22	361
	3:45	23	344	21	361	31	357	21	348	24	365	18	372	13	406		2553	21	364
	04:00	27	368	21	398	27	343	29	351	34	359 .	28	373	12	385		2577	25	368
	04:15	25	327	26	358	40	353	28	344	36	360	16	378	12	331		2451	26	350
	04:30	26	296	44	358	29	325	38	322	30	301	26	353	11	355		2310	29	330
	34:45	43	259	65	323	49	347	43	334	4.5	333	12	350	12	334		2280	38	325
	05:00	54	303	57	350	55	351	68	333	61	328	25	353	10	380		2398	47	342
	05:15	65	309	70	329	77	310	65	346	83	325	25	345	16	347	401	2311	57	330
	05:30	115	334	99	324	118	338	115	336	105	340	40	348	16	362		2382	86	340
	05:45	162	336	154	354	168	351	164	328	166	338	54	360	27	340		2407	127	343
	06:00	226	366	229	356	237	411	218	336	221	363	56	284	48	357		2473	176	353
	06:15	247	327	285	399	276	334	319	331	264	379	73	283	42	306		2359	215	337
	06:30	236	356	271	302	287	310	282	382	287	383	119	255	59	318		2306	220	329
	06:45	294	319	298	291	332	342	290	350	347	388	111	293	95	272		2255	252	322
	07:00	303	251	344	350	353	337	374	356	378	341	129	249	84	244		2128	280	304
	17:15	372	268	413	344	428	338	394	303	395	347	148	237	8.5	272		2109	319	301
	37:30	407	241	452	308	449	256	444	254	453	316	156	228	92	267		1870	350	267
	07:45	445	272	489	276	486	251	491	235	484	282	149	227	140	219		1762	383	251
	00:80	512	187	542	200	498	207	545	255	463	253	219	225	123	228		1555	414	222
	08:15	440	176	495	193	458	195	419	207	373	210	200	171	118	197		1349	357	192
	08:30	460	180	502	178	451	202	440	207	556	193	213	193	162	210		1363	397	194
	08:45	409	172	392	194	425	178	479	165	479	178	295	166	136	177		1230	373	175
	09:00	465	147	460	166 130	426	173 182	433	169	425	182	279	170	156	184	2691	1191 987	377	170 164
	09:15	461		489	158	472	185	480	184	412	195	302	193	170	158		1073	398	178
		432		477	168	444	207	442	197	360	197	286	183	219	156		1108	380	184
	09:45	369		407	102	449	153	396	153	392	168	340	159	240	134	2593	869	370	144
	10:15	346		324	108	264	184	362	166	347	181	356	186	249	110	2248	935	321	155
	10:30	295		351	103	298	130	346	118	364	152	326	182	290	131	2270	816	324	136
	10:45	326		339	125	311	110	356	133	364	150	360	175	318	116	2374	809	339	134
	11:00			354					104		153		183		84				119
	11:15			300	100	281	86	339			127		155		89				111
	11:30			313	100 79	294	73				109		131						94
	11:45			303	63	290	85		88		111		89				503		83
				304	65	329					108		98						78
																	5222		2361
AM AM	Times Peaks		7:45 1857		7:45		7:45 1893		8:45 1925		7:45		11:15		11:15		7:45 10873		7:45
																	14:30 10822		
PM	Peaks		1520		1590		1609		1589		1623		1638		1526		10822		1544

APPENDIX C

Crash Data

LOCATION 1

I-93 Southbound Between I-95 and Montvale Avenue in Woburn and Stoneham

						Total								
	Crash	Crash			Number of		Total Fata	ı	Road		Weather			
Count	Number	Year Crash Time	Crash Date	Crash Severity	Vehicles	Injury	Injury	Manner of Collision	Surface	Ambient Light Condition	Condition	Vehicle Traveled Direction	Roadway	Location
	1 2645141		23-Sep-2010	<u> </u>	2	0	0	Single vehicle crash	Dry	Daylight	Not Reported	V1:Southbound / V2:Southbound	Rte 93 S	Exit 36 on Rte 93 S
	2 2555994		18-Jan-2010	Property damage only (n	2	0	0	Sideswipe, same direction	Snow	Daylight	Not Reported	V1:Southbound / V2:Southbound	Rte 93 S	Exit 36 on Rte 93 S
	3 2567185		12-Feb-2010		2	0	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound	Rte 93 S	Exit 36 on Rte 93 S
	4 2590453		23-Mar-2010		3	0	0	Rear-end	Wet	Daylight	Cloudy/Rain	V1:Southbound / V2:Southbound / V		Exit 36 on Rte 93 S
	5 2591433		15-Apr-2010		2	0	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound		NTVALE AVENUE
	6 2598097	2010 4:16 AM	16-May-2010	Property damage only (n	1	0	0	Single vehicle crash	Dry	Dark - lighted roadway	Clear	V1:Southbound	Rte 93 S	Exit 36 on Rte 93 S
	7 2614147	2010 8:30 PM	22-Jun-2010	Non-fatal injury	1	1	0	Single vehicle crash	Other	Dark - lighted roadway	Not Reported	V1:Southbound	Rte 93 S	Exit 36 on Rte 93 S
	8 2670945	2010 11:26 PM	14-Dec-2010	Non-fatal injury	4	1	0	Sideswipe, same direction	Dry	Dark - lighted roadway	Cloudy	V1:Southbound / V2:Not reported /	V3 Rte 93 S	Exit 36 on Rte 93 S
	9 2649274	2010 7:57 AM	30-Sep-2010	Non-fatal injury	4	2	0	Rear-end	Dry	Daylight	Cloudy	V1:Southbound / V2:Southbound / V	/3: Rte 93 S	Exit 36 on Rte 93 S
-	10 2653156	2010 11:44 PM	18-Oct-2010	Non-fatal injury	1	1	0	Single vehicle crash	Dry	Dark - lighted roadway	Clear	V1:Southbound	Rte 93 S	Exit 36 on Rte 93 S
	11 2662901	2010 6:15 PM	08-Nov-2010	Non-fatal injury	2	1	0	Angle	Dry	Dark - roadway not lighted	Clear/Rain	V1:Southbound / V2:Southbound	Rte 93 S	Exit 36 on Rte 93 S
-	2663635	2010 9:25 AM	22-Nov-2010	. , , , , ,	4	0	0	Rear-end	Dry	Daylight	Not Reported	V1:Southbound / V2:Southbound / V	/3: Rte 93 S	Exit 36 on Rte 93 S
	13 2666188		01-Dec-2010	Non-fatal injury	1	1	0	Single vehicle crash	Wet	Daylight	Not Reported	V1:Southbound	RAMP-MON	TVALE AVE
:	14 2700876	2011 7:40 AM	17-Feb-2011	Property damage only (n	2	0	0	Rear-end	Dry	Daylight	Not Reported	V1:Southbound / V2:Southbound	Rte 93 S	Exit 36 on Rte 93 S
	15 2703014		03-Mar-2011	Property damage only (n	2	0	0	Rear-end	Dry	Dark - lighted roadway	Clear	V1:Southbound / V2:Southbound	Rte 93 S	Exit 36 on Rte 93
:	16 2728282	2011 11:38 PM	20-May-2011	Non-fatal injury	1	1	0	Sideswipe, same direction	Dry	Dark - lighted roadway	Clear	V1:Southbound	Rte 93 S	Exit 36 on Rte 93 S
- :	L7 2744167	2011 12:30 PM	24-Jul-2011	Non-fatal injury	2	2	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound	Rte 93 S	Exit 36 on Rte 93 S
:	L8 2778968	2011 10:28 PM	05-Oct-2011	Non-fatal injury	4	2	0	Rear-end	Dry	Dark - lighted roadway	Clear	V1:Southbound / V2:Southbound / V	/3: Rte 93 S	Exit 36 on Rte 93
- :	19 2812191	2011 5:33 PM	29-Nov-2011	Property damage only (n	2	0	0	Rear-end	Dry	Dark - lighted roadway	Cloudy	V1:Southbound / V2:Southbound	Rte 93 S	Exit 36 on Rte 93 S
2	20 2709812		24-Mar-2011	Property damage only (n	1	0	0	Single vehicle crash	Wet	Dark - lighted roadway	Snow	V1:Southbound	Rte 93 S	Exit 36 on Rte 93 S
2	21 2709816	2011 8:45 PM	25-Mar-2011	Non-fatal injury	3	1	0	Rear-end	Dry	Dark - lighted roadway	Clear	V1:Southbound / V2:Southbound / V	/3: Rte 93 S	Exit 36 on Rte 93 S
2	22 2718150	2011 3:31 PM	01-Apr-2011	Property damage only (n	2	0	0	Rear-end	Wet	Daylight	Rain	V1:Southbound / V2:Southbound		
2	23 2700863	2011 9:30 AM	20-Jan-2011	Property damage only (n	2	0	0	Rear-end	Wet	Daylight	Clear	V1:Southbound / V2:Southbound	Rte 93 S	Exit 36 on Rte 93 S
2	24 2702746	2011 1:59 AM	13-Feb-2011	Property damage only (n	1	0	0	Single vehicle crash	Dry	Dark - lighted roadway	Cloudy	V1:Southbound	Rte 93 S	Exit 36 on Rte 93 S
2	25 2716293	2011 8:25 PM	20-Apr-2011	Property damage only (n	4	0	0	Rear-end	Dry	Dark - lighted roadway	Clear	V1:Southbound / V2:Southbound / V	/3: Rte 93 S	Exit 36 on Rte 93 S
2	26 2721013	2011 7:15 PM	24-Apr-2011	Property damage only (n	2	0	0	Rear-end	Dry	Dusk	Clear	V1:Southbound / V2:Southbound	Rte 93 S	Exit 36 on Rte 93 S
2	27 2719038	2011 9:00 AM	26-Apr-2011	Non-fatal injury	3	1	0	Angle	Dry	Daylight	Clear	V1:Southbound / V2:Southbound / V	/3: Rte 93 S	Exit 36 on Rte 93 S
2	28 2727262	2011 9:10 PM	08-May-2011	Property damage only (n	2	0	0	Angle	Dry	Dark - lighted roadway	Clear	V1:Southbound / V2:Southbound	Rte 93 S	Exit 36 on Rte 93 S
2	29 2782648	2011 6:30 AM	07-Oct-2011	Property damage only (n	3	0	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound / V	/3: Rte 93 S	Exit 36 on Rte 93 S
3	30 2814845	2011 11:27 AM	31-Oct-2011	Property damage only (n	2	0	0	Angle	Dry	Daylight	Clear	V1:Southbound / V2:Westbound		
3	31 2941666	2011 11:15 PM	02-Dec-2011	Non-fatal injury	2	1	0	Sideswipe, same direction	Dry	Dark - lighted roadway	Not Reported	V1:Southbound / V2:Southbound	Rte 93 S	Exit 36 on Rte 93 S
3	3226326	2012 6:30 PM	01-Aug-2012	Non-fatal injury	3	1	0	Rear-end	Wet	Daylight	Clear	V1:Southbound / V2:Southbound / V	/3: Rte 93 S	Exit 36 on Rte 93 S
3	33 3243304	2012 00:00 AM	15-Aug-2012	Property damage only (n	2	0	0	Sideswipe, same direction	Dry	Dark - lighted roadway	Clear	V1:Southbound / V2:Southbound	Rte 93 S	Exit 36 on Rte 93
3	3248871	2012 6:36 AM	22-Aug-2012	Property damage only (n	4	0	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound / V	/3: Rte 93 S	Exit 36 on Rte 93
3	3278879	2012 9:48 AM	16-Oct-2012	Non-fatal injury	3	1	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound / V	/3: Rte 93	Exit 36 on Rte 93
3	3252668	2012 7:44 AM	14-Sep-2012	Property damage only (n	2	0	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound	Rte 93 S	Exit 36 on Rte 93 S
3	37 2951592	2012 7:30 PM	27-Feb-2012	Non-fatal injury	2	1	0	Sideswipe, same direction	Dry	Dark - lighted roadway	Not Reported	V1:Southbound / V2:Southbound	Rte 93 S	Exit 36 on Rte 93 S
3	3321446		13-Dec-2012	Property damage only (n	2	0	0	Rear-end	Dry	Dark - lighted roadway	Not Reported	V1:Southbound / V2:Southbound	Rte 93 S	Exit 36 on Rte 93 S
3	39 2853093		05-Jan-2012	Property damage only (n	2	0	0	Rear-end	Dry	Dark - lighted roadway	Clear	V1:Southbound / V2:Southbound	Rte 93 S	Exit 36 on Rte 93 S
4	10 2872386	2012 6:52 AM	13-Jan-2012	Property damage only (n	2	0	0	Rear-end	Dry	Dark - lighted roadway	Clear	V1:Southbound / V2:Southbound	Rte 93 S	Exit 36 on Rte 93 S
4	11 2894055	2012 5:45 PM	27-Jan-2012	Property damage only (n	2	0	0	Rear-end	Wet	Dark - lighted roadway	Rain	V1:Southbound / V2:Southbound	Rte 93 S	Exit 36 on Rte 93 S
4	12 2894059	2012 7:25 AM	01-Feb-2012	Property damage only (n	3	0	0	Rear-end	Dry	Daylight	Not Reported	V1:Southbound / V2:Southbound / V	/3: Rte 93 S	Exit 36 on Rte 93 S
4	13 2914850	2012 6:20 PM	09-Feb-2012	Property damage only (n	2	0	0	Rear-end	Dry	Dark - lighted roadway	Not Reported	V1:Southbound / V2:Southbound	Rte 93 S	Exit 36 on Rte 93 S
4	14 2937668	2012 7:50 PM	01-Mar-2012	Property damage only (n	4	0	0	Angle	Snow	Dark - lighted roadway	Snow	V1:Southbound / V2:Southbound / V	/3: Rte 93 S	Exit 36 on Rte 93 S
4	15 2976134	2012 8:00 AM	19-Mar-2012	Non-fatal injury	2	1	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound	Rte 93 S	Exit 36 on Rte 93 S
4	16 3018135	2012 8:27 AM	03-Apr-2012	Property damage only (n	2	0	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound	Rte 93 S	Exit 36 on Rte 93 S
	17 3019528	2012 3:30 AM	09-Apr-2012	Non-fatal injury	2	1	0	Rear-end	Dry	Dark - lighted roadway	Clear	V1:Southbound / V2:Southbound	Rte 93 S	Exit 36 on Rte 93 S
4	18 3207184	2012 6:21 AM	23-Jul-2012	Property damage only (n	2	0	0	Sideswipe, same direction	Wet	Daylight	Cloudy/Rain	V1:Southbound / V2:Southbound	Rte 93 S	Exit 36 on Rte 93 S
4	19 3245395	2012 00:00 AM	24-Aug-2012	Property damage only (n	1	0	0	Single vehicle crash	Dry	Dark - lighted roadway	Clear	V1:Southbound	Rte 93 S	Exit 36 on Rte 93 S
į	3266539	2012 7:44 AM	20-Sep-2012	Property damage only (n	2	0	0	Sideswipe, same direction	Dry	Daylight	Clear	V1:Southbound / V2:Southbound	Rte 93 S	Exit 36 on Rte 93 S
Ţ	3278621		14-Oct-2012		2	0	0	Angle	Dry	Dark - lighted roadway	Cloudy	V1:Southbound / V2:Southbound	Rte 93 S	Exit 36 on Rte 93 S
Į.	3286145		22-Oct-2012	Property damage only (n	2	0	0	Sideswipe, same direction	Dry	Daylight	Clear	V1:Southbound / V2:Southbound		
Ţ	3282338		24-Oct-2012	Non-fatal injury	5	2	0	Rear-end	Dry	Dark - roadway not lighted	Clear	V1:Southbound / V2:Southbound / V	/3: Rte 93 S	Exit 36 on Rte 93 S
	3285757			Property damage only (n	2	0	0	Rear-end	Dry	Dawn	Clear	V1:Southbound / V2:Southbound	Rte 93 S	Exit 36 on Rte 93 S
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								Total								
	Cra	ash	Crash				Number of	Nonfatal	Total Fata	l .	Road		Weather			
Count	Nu	mber	Year	Crash Time	Crash Date	Crash Severity	Vehicles	Injury	Injury	Manner of Collision	Surface	Ambient Light Condition	Condition	Vehicle Traveled Direction	Roadway	Location
	55	3290936	2	.012 6:25 AM	14-Nov-2012	Property damage only (n	2	0	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound	Rte 93 S	Exit 36 on Rte 93 S
	56	3293154	2	012 6:12 AM	19-Nov-2012	Property damage only (n	2	0	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound	Rte 93 S	Exit 36 on Rte 93 S
	57	3325690	2	012 2:03 AM	30-Dec-2012	Non-fatal injury	1	1	0	Single vehicle crash	Snow	Dark - lighted roadway	Snow	V1:Southbound	Rte 93 S	Exit 36 on Rte 93 S
	58 3	378131*	2	012 3:26 AM	01-Apr-2012	Fatal injury	2	1	1	Rear-end	Dry	Dark - lighted roadway	Clear	V1:Southbound / V2:Southbound	RTE 95	
	59 3	378030*	2	012 4:19 AM	01-Apr-2012	Property damage only (n	3	0	0	Rear-end	Dry	Dark - lighted roadway	Clear	V1:Southbound / V2:Southbound / V3:	: RTE 95	
	60 3	3123331*	2	012 8:35 AM	06-Jun-2012	Not Reported	4	0	0	Rear-end	Dry	Daylight	Cloudy	V1:Southbound / V2:Southbound / V3:	S/ OF RT 128	
	61 3	154571*	2	012 8:15 AM	15-Jun-2012	Non-fatal injury	2	1	0	Rear-end	Dry	Daylight	Not Reported	V1:Southbound / V2:Southbound		Exit 37 on Rte 93 S

LOCATION 2

I-95 Southbound at the I-90 Interchange in Weston

Count	Crash Number	Crash Year Crash Time	Crash Date	Crash Severity	Number of Vehicles	Total Nonfatal	Total Fatal Injury	Manner of Collision	Road Surface	Ambient Light	Weather Condition	Vehicle Traveled Direction	Roadway	Location
1	2641964	4 2010 7:45 AM	24-Aug-2010	Non-fatal injury	1	1	0	Single vehicle crash	Wet	Daylight	Cloudy/Rain	V1:Southbound	Rte 95 S	Exit 24 on Rte 95 S
2			01-Jun-2010	Property damage only (no	3	0	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound / V3:Sou		Exit 25 on Rte 95
3				Property damage only (no	2	0	0	Rear-end	Dry	Daylight	Not Reported	V1:Southbound / V2:Southbound	Rte 95 S	Exit 25 on Rte 95 S
4	2618905		18-Jun-2010		3	0	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound / V3:Sou		Exit 25 on Rte 95 S
5	2612071 2634065		19-Aug-2010	Property damage only (no Property damage only (no	3	0	0	Single vehicle crash Rear-end	Dry	Daylight Daylight	Clear	V1:Southbound V1:Southbound / V2:Southbound / V3:Sou	Rte 95 S	Exit 24 on Rte 95 S Exit 24 on Rte 95 S
7			19-Aug-2010 19-Mar-2010		2	1	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound	Rte 95 S	Exit 25 on Rte 95 S
8	2592131			Non-fatal injury	2	1	0	Sideswipe, same direction	Dry	Daylight	Cloudy	V1:Southbound / V2:Southbound	Rte 95 S	RAMP-RT 90 TO RT 95 SB
9			06-Jul-2010	Non-fatal injury	1	1	0	Single vehicle crash	Dry	Daylight	Clear	V1:Southbound	Rte 95 S	Exit 24 on Rte 95 S
10	2624720	2010 2:27 PM	23-Jul-2010	Non-fatal injury	2	1	0	Rear-end	Dry	Daylight	Cloudy	V1:Southbound / V2:Southbound	Rte 95 S	Exit 25 on Rte 95 S
11			26-Aug-2010	Non-fatal injury	4	1	0	Rear-end	Dry	Daylight	Not Reported	V1:Southbound / V2:Southbound / V3:Sou	utł Rte 95 S	Exit 25 on Rte 95 S
12			04-Oct-2010		2	1	0	Rear-end	Wet	Daylight	Cloudy/Rain	V1:Southbound / V2:Southbound	Rte 95 S / Rte 30	RAMP-RT 30 TO RT 95 SB
13				Property damage only (no	2	0	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound	Rte 95	Exit 25 on Rte 95
14 15	2656562 2663166			Non-fatal injury Property damage only (no	1 4	0	0	Single vehicle crash Rear-end	Dry Dry	Daylight Daylight	Clear Cloudy	V1:Southbound V1:Southbound / V2:Southbound / V3:Sou	Rte 95 S	Exit 24 on Rte 95 S Exit 25 on Rte 95 S
16				Non-fatal injury	2	1	0	Rear-end	Dry	Daylight Dark - roadway not lighted	Not Reported	V1:Southbound / V2:Southbound	Rte 95 S	Exit 25 on Rte 95 S
17				Non-fatal injury	1	1	0	Rear-end	Dry	Daylight	Clear	V1:Southbound	Rte 95 S	Exit 24 on Rte 95 S
18	2765897	7 2011 6:24 AM		Property damage only (no	2	0	0	Sideswipe, same direction	Dry	Dawn	Fog, smog, smoke	V1:Southbound / V2:Southbound	Rte 95	Exit 24 on Rte 95
19	2835781	1 2011 4:00 PM	09-Dec-2011	Property damage only (no	2	0	0	Rear-end	Dry	Dusk	Clear	V1:Southbound / V2:Southbound	Rte 95 S	Exit 24 on Rte 95 S
20	2680997	7 2011 4:58 PM	11-Jan-2011	Non-fatal injury	3	1	0	Rear-end	Dry	Dusk	Clear	V1:Southbound / V2:Southbound / V3:Sou	utł Rte 95 S	Exit 25 on Rte 95 S
21				Property damage only (no	2	0	0	Angle	Dry	Daylight	Clear	V1:Southbound / V2:Southbound	Rte 95 S	Exit 25 on Rte 95 S
22				Non-fatal injury	1	1	0	Single vehicle crash	Dry	Daylight	Clear	V1:Southbound	Rte 95 S	RAMP-RT 90 TO RT 95 SB
23				Property damage only (no	4	0	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound / V3:Sou		Exit 25 on Rte 95 S
24 25	2727714 2728134			Property damage only (no Non-fatal injury	2 3	1	0	Angle Sideswipe, same direction	Dry	Daylight Daylight	Not Reported Clear	V1:Southbound / V2:Southbound V1:Southbound / V2:Southbound / V3:Sou	Rte 95 S	Exit 25 on Rte 95 S Exit 25 on Rte 95 S
26	2727785			Non-fatal injury	3	1	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound / V3:Sou		Exit 25 on Rte 95 S
27				Property damage only (no	2	0	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound	Rte 95 S	Exit 25 on Rte 95 S
28	2737919		20-Jun-2011		2	1	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound	Rte 95 S	Exit 25 on Rte 95 S
29	2786844	4 2011 7:40 PM	22-Jun-2011	Property damage only (no	3	0	0	Rear-end	Dry	Dusk	Clear	V1:Southbound / V2:Southbound / V3:Sou	utł Rte 95 S	Exit 25 on Rte 95 S
30	2738606	5 2011 3:39 PM	28-Jun-2011	Non-fatal injury	3	2	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound / V3:Sou	utł Rte 95 S	Exit 25 on Rte 95 S
31			14-Jul-2011	Property damage only (no	2	0	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound	Rte 95 S	Exit 25 on Rte 95 S
32			20-Jul-2011	Property damage only (no	3	0	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound / V3:Sou		Exit 25 on Rte 95 S
33	2751662 2750709		21-Jul-2011	Property damage only (no	3	1	0	Rear-end	Dry	Daylight	Not Reported Clear	V1:Southbound / V2:Southbound / V3:Sou V1:Southbound / V2:Southbound / V3:Sou		Exit 24 on Rte 95 S
35			11-Aug-2011	Non-fatal injury Non-fatal injury	1	1	0	Rear-end Single vehicle crash	Dry Wet	Daylight Daylight	Rain	V1:Southbound / V2:Southbound / V3:Sot V1:Southbound	Rte 95 S	Exit 25 on Rte 95 S Exit 25 on Rte 95 S
36	2782652		05-Oct-2011		3	2	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound / V3:Sou		Exit 25 on Rte 95 S
37	2787675		07-Oct-2011		3	0	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound / V3:Sou		Exit 24 on Rte 95 S
38	2786846			Property damage only (no	4	0	0	Rear-end	Dry	Daylight	Cloudy	V1:Southbound / V2:Southbound / V3:Sou		Exit 25 on Rte 95 S
39	2793997	7 2011 5:20 PM	09-Nov-2011	Not Reported	1	0	0	Single vehicle crash	Dry	Dark - roadway not lighted	Clear	V1:Southbound	Rte 95 S	Exit 25 on Rte 95 S
40	2793998			Property damage only (no	4	0	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound / V3:Sou		Exit 25 on Rte 95 S
41				Property damage only (no	2	0	0	Rear-end	Dry	Dusk	Clear	V1:Southbound / V2:Southbound	Rte 95 S	Exit 24 on Rte 95 S
42	2805793		22-Nov-2011		2	0	0	Rear-end	Dry	Dark - roadway not lighted	Clear	V1:Southbound / V2:Southbound	Rte 95 S	Exit 25 on Rte 95 S
43				Not Reported	1 4	0	0	Single vehicle crash	Dry Dry	Daylight	Cloudy	V1:Southbound V1:Southbound / V2:Southbound / V3:Sou	Rte 95 S	Exit 24 on Rte 95 S
44				Property damage only (no Property damage only (no	2	0	0	Rear-to-rear Rear-end	Dry	Dark - roadway not lighted Daylight	Clear	V1:Southbound / V2:Southbound	Rte 95 S	Exit 25 on Rte 95 S Exit 25 on Rte 95 S
46	2954168			Property damage only (no	1	0	0	Sideswipe, same direction	Wet	Daylight	Rain	V1:Southbound	Rte 95 S	Exit 25 on Rte 95 S
47				Not Reported	2	0	0	Angle	Dry	Daylight	Clear	V1:Southbound / V2:Southbound	Rte 95 S	Exit 25 on Rte 95 S
48	3381262			Property damage only (no	1	0	0	Single vehicle crash	Wet	Dark - unknown roadway ligi		V1:Southbound		
49	2889040	2012 11:35 PM		Property damage only (no	1	0	0	Single vehicle crash	Snow	Dark - roadway not lighted	Snow	V1:Southbound	Rte 95	Exit 25 on Rte 95
50	2896829			Property damage only (no	1	0	0	Single vehicle crash	Dry	Dark - lighted roadway	Clear	V1:Southbound	Rte 95 S	Exit 24 on Rte 95 S
51				Non-fatal injury	3	3	0	Rear-end	Dry	Dusk	Clear	V1:Southbound / V2:Southbound / V3:Sou		Exit 25 on Rte 95 S
52			· · · · · · · · · · · · · · · · · · ·	Not Reported	2	0	0	Rear-end	Dry	Dusk	Clear	V1:Southbound / V2:Southbound	Rte 95 S	Exit 25 on Rte 95 S
53 54	3049845 3068814			Non-fatal injury Not Reported	2 2	0	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound	Rte 95 S Rte 95 S	Exit 24 on Rte 95 S Exit 25 on Rte 95 S
55			· · · · · · · · · · · · · · · · · · ·	Not Reported Not Reported	2	0	0	Rear-end Rear-end	Dry Wet	Daylight Daylight	Not Reported	V1:Southbound / V2:Southbound V1:Southbound / V2:Southbound	Rte 95 S	Exit 25 on Rte 95 S
56			•	Not Reported	2	0	0	Angle	Dry	Daylight	Not Reported	V1:Southbound / V2:Southbound	Rte 95 S	Exit 24 on Rte 95 S
57				Non-fatal injury	2	2	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound	Rte 95 S	, , , , , , , , , , , , , , , , , , , ,
58	3158785			Property damage only (no	1	0	0	Single vehicle crash	Wet	Dawn	Rain	V1:Southbound	Rte 95 S	Exit 24 on Rte 95 S
59	3163119	9 2012 11:00 AM	21-Jun-2012	Property damage only (no	3	0	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound / V3:No	rtl Rte 95 S	Exit 25 on Rte 95 S
60	3168335		08-Jul-2012		4	3	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound / V3:Sou		Exit 25 on Rte 95 S
61				Non-fatal injury	2	1	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound	Rte 95 S	Exit 25 on Rte 95 S
62	3235165			Property damage only (no	2	0	0	Rear-end	Wet	Daylight	Cloudy/Rain	V1:Southbound / V2:Southbound	Rte 95 S	Exit 25 on Rte 95 S
63				Property damage only (no	<u>2</u>	3	0	Rear-end	Dry	Daylight	Cloudy	V1:Southbound / V2:Southbound	Rte 95 S	Exit 25 on Rte 95 S
64	3248907 3247322			Non-fatal injury Property damage only (no		0	0	Rear-end Rear-end	Dry Dry	Daylight Daylight	Clear	V1:Southbound / V2:Southbound / V3:Sou V1:Southbound / V2:Southbound	Rte 95 S	Exit 25 on Rte 95 S Exit 25 on Rte 95 S
66				Property damage only (no	2	0	0	Sideswipe, same direction	Dry	Daylight Dark - lighted roadway	Clear	V1:Southbound / V2:Southbound V1:Southbound / V2:Southbound	Rte 95 S	Exit 25 on Rte 95 S
67				Property damage only (no		0	0	Sideswipe, same direction	Dry	Dark - lighted roadway	Clear	V1:Southbound / V2:Southbound	Rte 95 S	RAMP-RT 90 TO RT 95 SB
68	3286163			Non-fatal injury	2	1	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound	Rte 95 S	Exit 25 on Rte 95 S
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Count	Crash Number	Crash Year Crash Time	Crash Date	Crash Severity	Number of Vehicles	Total Nonfatal	Total Fatal Injury	Manner of Collision	Road Surface	Ambient Light	Weather Condition	Vehicle Traveled Direction	Roadway	Location
	69 3284865	5 2012 9:00 AM	30-Oct-2012	Non-fatal injury	1	1	0	Single vehicle crash	Water (standing,	r Daylight	Rain	V1:Southbound	Rte 95 S	Exit 25 on Rte 95 S
	70 3292327	7 2012 5:40 PM	16-Nov-2012	Property damage only (no	2	0	0	Rear-end	Dry	Dark - roadway not lighted	Clear	V1:Southbound / V2:Southbound	Rte 95 S	Exit 24 on Rte 95 S
	71 3381270	2012 9:59 AM	06-Dec-2012	Property damage only (no	2	0	0	Rear-end	Dry	Daylight	Clear	V1:Southbound / V2:Southbound		
	72 3301816	5 2012 4:40 PM	06-Dec-2012	Non-fatal injury	3	1	0	Rear-end	Dry	Dark - roadway not lighted	Not Reported	V1:Southbound / V2:Southbound / V3:Sout	ł Rte 95 S	Exit 25 on Rte 95 S
	73 3309434	4 2012 12:22 PM	08-Dec-2012	Non-fatal injury	1	1	0	Single vehicle crash	Wet	Daylight	Cloudy	V1:Southbound	Rte 95 S	Exit 24 on Rte 95 S
	74 3310225	5 2012 4:44 PM	12-Dec-2012	Property damage only (no	2	0	0	Rear-end	Dry	Dark - lighted roadway	Clear	V1:Southbound / V2:Southbound	Rte 95 S	Exit 25 on Rte 95 S
	75 3321378	8 2012 5:40 PM	17-Dec-2012	Property damage only (no	3	0	0	Rear-end	Wet	Dark - roadway not lighted	Rain	V1:Southbound / V2:Southbound / V3:Sout	rł Rte 95 S	Exit 25 on Rte 95 S
	76 3321474	4 2012 5:59 PM	19-Dec-2012	Property damage only (no	2	0	0	Rear-end	Dry	Dark - lighted roadway	Not Reported	V1:Southbound / V2:Southbound	Rte 95 S	Exit 25 on Rte 95 S
	77 3381273	3 2012 10:39 PM	25-Dec-2012	Property damage only (no	1	0	0	Single vehicle crash	Wet	Dark - roadway not lighted	Clear	V1:Southbound		RAMP-RT 30 TO RT 95 SB

APPENDIX D

Level of Service (LOS) Analysis
Freeway Ramp Merge and Diverge Analys s

LOCATION 1

I-93 Southbound Between I-95 and Montvale Avenue in Woburn and Stoneham

2015 Existing Conditions

- 1. 2015 AM Merge
- 2. 2015 AM Diverge
- 3. 2015 AM Basis Freeway

Fax:

Phone:

E-mail: _____Merge Analysis____ Seth Analyst: Agency/Co.: CTPS Date performed: 5/20/2015
Analysis time period: AM Peak Hour Freeway/Dir of Travel: I-93 SB (I-95 to Montvale Ave) Junction: Merge from I-95 NB to I-93 SB

Jurisdiction: Highway District 4

Analysis Year: 2015 Existing Description: Low-Cost Improvements to Bottleneck Locations _____Freeway Data_____ Type of analysis Merge Number of lanes in freeway 4 65.0 5750 Free-flow speed on freeway mph Volume on freeway vph On Ramp Data____ Side of freeway Right Number of lanes in ramp 1 Free-flow speed on ramp 30.0 mph Volume on ramp 1450 vph Length of first accel/decel lane 1500 ft Length of second accel/decel lane _____Adjacent Ramp Data (if one exists)_____ Yes 850 Does adjacent ramp exist? Volume on adjacent Ramp vph Position of adjacent Ramp Upstream Type of adjacent Ramp Off 1000 ft Distance to adjacent Ramp _____Conversion to pc/h Under Base Conditions_____ Junction Components Freeway Ramp Adjacent Ramp
5750 1450 850
0.95 0.95 0.95
1513 382 224
4 3 3 3
0 0 0 0
Level Level Level
% %
mi mi
1.5 1.5 1.5
1.2 1.2 1.2 Volume, V (vph) vph Peak-hour factor, PHF Peak 15-min volume, v15 Trucks and buses Recreational vehicles Terrain type: Grade Length mi Trucks and buses PCE, ET Recreational vehicle PCE, ER

```
6300
Flow rate, vp
                                             1581
                                                        927
                                                                 pcph
                     Estimation of V12 Merge Areas
                L =
                              (Equation 13-6 or 13-7)
                ΕQ
                      0.020 Using Equation 0
                FM
                v = v (P) = 127
                                    pc/h
                 12 F FM
                        Capacity Checks
                                                  LOS F?
                        Actual
                                    Maximum
                        7881
                                    9400
                                                   No
    V
     FO
                        3086 pc/h
                                   (Equation 13-14 or 13-17)
    v or v
          av34
     3
               > 2700 pc/h?
Ιs
    v or v
                                    Yes
     3
         av34
                > 1.5 v /2
Ιs
    v or v
                                    Yes
          av34
                  12
     3
If yes, v
          = 2520
                                  (Equation 13-15, 13-16, 13-18, or 13-19)
       12A
                    Flow Entering Merge Influence Area
                   Actual Max Desirable
                                                   Violation?
                                4600
                   4101
                                                    No
     12A
              Level of Service Determination (if not F)
Density, D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 27.3 pc/mi/ln
                                     12
Level of service for ramp-freeway junction areas of influence C
                  Speed Estimation
Intermediate speed variable,
                                        M = 0.467
                                        S
Space mean speed in ramp influence area,
                                        S = 54.3
                                                    mph
                                        R
Space mean speed in outer lanes,
                                        S = 60.0
                                                    mph
                                        0
Space mean speed for all vehicles,
```

S = 56.9

mph

0.980

0.98

0.985

0.98

0.985

0.98

Heavy vehicle adjustment, fHV

Driver population factor, fP

Phone: E-mail:

Fax:

_____Diverge Analysis______

Analyst: Seth Asante

Agency/Co.:

Date performed: 5/20/2015
Analysis time period: AM Peak Hour

Freeway/Dir of Travel: I-93 SB (I-95 to Montvale Ave) Junction:

Jurisdiction:

Analysis Year:

Diverge I-93 SB to Montvale
Highway District 4
2015 Existing

Description: Low-Cost Improvement to Bottleneck Locations

F	reeway	Data
---	--------	------

Diverge	
4	
65.0	mph
6300	vph
	4 65.0

_____Off Ramp Data_____

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	30.0	mph
Volume on ramp	1400	vph
Length of first accel/decel lane	350	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

vph

Does adjacent ramp exist? Yes 150 Volume on adjacent ramp Position of adjacent ramp Downstream Type of adjacent ramp On

Distance to adjacent ramp 700 ft

(Conversion	τo	pc/n	unaer	Base	Conditions	
			-			_	

Junction Components	Freeway	Ramp	Adjacent
			Ramp
Volume, V (vph)	6300	1400	150 vph
Peak-hour factor, PHF	0.95	0.95	0.95
Peak 15-min volume, v15	1658	368	39 v
Trucks and buses	4	3	3 %
Recreational vehicles	0	0	0 %
Terrain type:	Level	Level	Level
Grade	0.00 %	0.00 %	0.00 %
Length	0.00 mi	0.00 mi	0.00 mi
Trucks and buses PCE, ET	3.0*	3.0*	3.0*
Recreational vehicle PCE, ER	1.2	1.2	1.2

```
Driver population factor, fP
                                     0.98
                                                0.98
                                                            0.98
                                     7308
Flow rate, vp
                                                1594
                                                            171
                                                                     pcph
                      Estimation of V12 Diverge Areas
                 L =
                                (Equation 13-12 or 13-13)
                 ΕQ
                        0.436 Using Equation 0
                 FD
                 v = v + (v - v) P = 4085 pc/h
                           F R FD
                  12
                     R
                          Capacity Checks
                                                      LOS F?
                         Actual
                                       Maximum
                         7308
                                       9400
                                                      No
    v = v
     Fi F
                         5714
                                       9400
                                                      No
    v = v - v
     FO
          \mathbf{F}
            R
                         1594
                                       2000
                                                      No
     R
    v or v
                         1611 pc/h (Equation 13-14 or 13-17)
     3
          av34
Ιs
    v or v
               > 2700 pc/h?
                                      No
     3
           av34
                > 1.5 v /2
Ts
    v or v
                                       No
     3
          av34
                       12
                                    (Equation 13-15, 13-16, 13-18, or 13-19)
If yes, v = 4085
        12A
                     Flow Entering Diverge Influence Area
                                                       Violation?
                     Actual
                                  Max Desirable
                     4085
                                  4400
                                                       No
    v
     12
               Level of Service Determination (if not F)
                      D = 4.252 + 0.0086 \text{ v} - 0.009 \text{ L} = 36.2 \text{ pc/mi/ln}
Density,
                      R
                                         12 D
Level of service for ramp-freeway junction areas of influence E
                Speed Estimation
                                           D = 0.636
Intermediate speed variable,
                                           S
Space mean speed in ramp influence area,
                                          S = 50.4
                                                       mph
                                           R
Space mean speed in outer lanes,
                                           S = 68.9
                                                       mph
                                           0
```

S = 57.1

mph

0.926

0.943

0.943

Heavy vehicle adjustment, fHV

Space mean speed for all vehicles,

Phone: E-mail:		Fax:		
	Operational An	alysis		
Analyst: Agency or Company: Date Performed: Analysis Time Period: Freeway/Direction: From/To: Jurisdiction: Analysis Year: Description: Low-Cost	Seth CTPS 8/31/2015 AM Peak Hour I-93 Southbound I-95 to Montval Highway Distric 2015 Improvements to	<mark>e Avenue</mark> t 4	tions	
	Flow Inputs an	d Adjustments		
Volume, V Peak-hour factor, PHF Peak 15-min volume, v15 Trucks and buses Recreational vehicles		7200 0.95 1895 4 0	veh/h v % %	
Terrain type: Grade Segment length Trucks and buses PCE, E Recreational vehicle PC Heavy vehicle adjustmen Driver population facto	E, ER t, fHV	Level - - 1.5 1.2 0.980 0.98	% mi	
Flow rate, vp	-,	1972	pc/h/ln	
Speed Inputs and Adjustments				
Lane width Right-side lateral clea Total ramp density, TRD Number of lanes, N Free-flow speed: FFS or BFFS Lane width adjustment, Lateral clearance adjus TRD adjustment Free-flow speed, FFS	fLW	- - 4 Measured 65.0 - - - 65.0	<pre>ft ft ramps/mi mi/h mi/h mi/h mi/h mi/h mi/h mi/h</pre>	
riee ilow speed, ris			1111/11	
	LOS and Perfor	mance Measures_		
Flow rate, vp Free-flow speed, FFS Average passenger-car s Number of lanes, N Density, D Level of service, LOS	peed, S	1972 65.0 60.4 4 32.7	pc/h/ln mi/h mi/h pc/mi/ln	

D

Level of service, LOS

2015 Existing Conditions

- 1. 2015 PM Merge
- 2. 2015 PM Diverge
- 3. 2015 PM Basis Freeway

Fax:

Phone:

E-mail:

E-Mail:				
Mer	ge Analysis			
Analyst: Agency/Co.: Date performed: Analysis time period: Freeway/Dir of Travel: Junction: Jurisdiction: Analysis Year: Description: Low-Cost Seth Asante CTPS 5/20/2015 PM Peak Hour I-93 SB (I-9) Merge from I-93 SB (I-9	<mark>5 to Montvale</mark> -95 NB to I-9 rict 4 g)	3 SB	ns	
Fr	eeway Data			
Type of analysis Number of lanes in freeway Free-flow speed on freeway Volume on freeway	Merge 4 65.0 5950		mph vph	
On	Ramp Data			
Side of freeway Number of lanes in ramp Free-flow speed on ramp Volume on ramp Length of first accel/decel lane Length of second accel/decel lane	Right 1 30.0 1700 1500		mph vph ft ft	
Adjacent Rai	mp Data (if o	ne exist	s)	
Does adjacent ramp exist? Volume on adjacent Ramp Position of adjacent Ramp Type of adjacent Ramp Distance to adjacent Ramp	Yes 950 Upstr Off 1000	eam	vph ft	
Conversion to pc	/h Under Base	Conditi	ons	
Junction Components Volume, V (vph) Peak-hour factor, PHF Peak 15-min volume, v15 Trucks and buses Recreational vehicles Terrain type:	Freeway 5950 0.95 1566 4 0 Level	Ramp 1700 0.95 447 3 0 Level	F 9 0 2 3	Adjacent Ramp 950 vph 0.95 250 v 3 % Cevel
Grade Length Trucks and buses PCE, ET Recreational vehicle PCE, ER	% mi 1.5 1.2		% mi 1	% mi 1.5

```
6519
                                           1853
                                                    1036
Flow rate, vp
                                                             pcph
                  Estimation of V12 Merge Areas
               L =
                             (Equation 13-6 or 13-7)
               ΕQ
               P = -0.014 Using Equation 0
               FM
               v = v (P) = -89 pc/h
                12 F FM
                       Capacity Checks
                                               LOS F?
                       Actual
                                  Maximum
                       8372
                                   9400
                                                No
    V
    FO
    v or v
                      3304 pc/h (Equation 13-14 or 13-17)
    3
         av34
    v or v
              > 2700 pc/h?
Ιs
                                  Yes
    3 av34
    v or v
               > 1.5 v /2
                                 Yes
Ιs
                12
3 	 av34
If yes, v = 2607
                                (Equation 13-15, 13-16, 13-18, or 13-19)
      12A
                   Flow Entering Merge Influence Area
                  Actual Max Desirable Violation?
                  4460
                              4600
                                                 No
     12A
            Level of Service Determination (if not F)
Density, D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 30.0 pc/mi/ln
                                  12
Level of service for ramp-freeway junction areas of influence D
                 Speed Estimation
Intermediate speed variable,
                                      M = 0.568
                                      S
Space mean speed in ramp influence area,
                                      S = 51.9
                                                mph
                                      R
Space mean speed in outer lanes,
                                      S = 59.8
                                                 mph
                                      0
```

S = 55.3

mph

0.980

0.98

0.985

0.98

0.985

0.98

Heavy vehicle adjustment, fHV

Driver population factor, fP

Space mean speed for all vehicles,

Phone: E-mail: Fax:

______Diverge Analysis______

Analyst: Seth Asante

Agency/Co.: CTPS Date performed: 5/20/2015
Analysis time period: PM Peak Hour

Freeway/Dir of Travel: I-93 SB (I-95 to Montvale Ave) Junction:

Diverge I-93 SB to Montvale

Highway District 4

Analysis Year:

2015 Existing

Description: Low-Cost Improvements to Bottleneck Locations

Freeway	L)a	ta	L
---------	---	----	----	---

Diverge	
4	
65.0	mph
7650	vph
	4 65.0

Off Ramp Data_____

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	30.0	mph
Volume on ramp	1000	vph
Length of first accel/decel lane	350	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? Yes 350 Volume on adjacent ramp vph Position of adjacent ramp Downstream Type of adjacent ramp On Distance to adjacent ramp 700 ft

Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent
			Ramp
Volume, V (vph)	7650	1000	350 vph
Peak-hour factor, PHF	0.94	0.94	0.94
Peak 15-min volume, v15	2035	266	93 v
Trucks and buses	4	4	3 %
Recreational vehicles	0	0	0 %
Terrain type:	Level	Level	Level
Grade	0.00 %	0.00 %	0.00 %
Length	0.00 mi	0.00 mi	0.00 mi
Trucks and buses PCE, ET	3.0*	3.0*	3.0*
Recreational vehicle PCE, ER	1.2	1.2	1.2

```
Driver population factor, fP
                                     0.98
                                                0.98
                                                            0.98
                                     8969
Flow rate, vp
                                                1172
                                                            403
                                                                     pcph
                      Estimation of V12 Diverge Areas
                 L =
                                (Equation 13-12 or 13-13)
                 ΕQ
                        0.436 Using Equation 0
                 FD
                 v = v + (v - v) P = 4571 pc/h
                           F R FD
                  12
                     R
                          Capacity Checks
                                                     LOS F?
                         Actual
                                       Maximum
                         8969
                                       9400
                                                      No
    v = v
     Fi F
                         7797
                                       9400
                                                      No
    v = v - v
     FO
          \mathbf{F}
            R
                         1172
                                       2000
                                                      No
     R
    v or v
                         2199 pc/h (Equation 13-14 or 13-17)
     3
         av34
Ιs
    v or v
               > 2700 pc/h?
                                      No
     3
          av34
                > 1.5 v /2
Ts
    v or v
                                       No
     3
          av34
                       12
If yes, v = 4571
                                    (Equation 13-15, 13-16, 13-18, or 13-19)
        12A
                     Flow Entering Diverge Influence Area
                     Actual
                                  Max Desirable
                                                       Violation?
                     4571
                                  4400
                                                       Yes
    v
     12
               Level of Service Determination (if not F)
                      D = 4.252 + 0.0086 \text{ v} - 0.009 \text{ L} = 40.4 \text{ pc/mi/ln}
Density,
                      R
                                         12 D
Level of service for ramp-freeway junction areas of influence E
                 Speed Estimation
                                           D = 0.598
Intermediate speed variable,
                                           S
Space mean speed in ramp influence area,
                                          S = 51.2
                                                       mph
                                           R
Space mean speed in outer lanes,
                                           S = 66.6
                                                       mph
                                           0
```

S = 57.8

mph

0.926

0.926

0.943

Heavy vehicle adjustment, fHV

Space mean speed for all vehicles,

Phone: E-mail:		Fax:	
Or	perational Analy	sis	
Analysis Time Period: PM Freeway/Direction: I-9 From/To: I-9	PS 31/2015 Peak Hour 93 Southbound 95 to Montvale A ghway District 4		ns
F]	ow Inputs and A	djustments	
Volume, V Peak-hour factor, PHF Peak 15-min volume, v15 Trucks and buses		7650 0.95 2013 4	veh/h v %
Recreational vehicles Terrain type: Grade Segment length		0 Level -	% % mi
Trucks and buses PCE, ET Recreational vehicle PCE, E Heavy vehicle adjustment, t Driver population factor, t	HV	1.5 1.2 0.980 0.98	
Flow rate, vp	peed Inputs and	2095	pc/h/ln
	occa inputs and		
Lane width Right-side lateral clearand Total ramp density, TRD Number of lanes, N Free-flow speed:	ce	- - - 4 Measured	ft ft ramps/mi
FFS or BFFS Lane width adjustment, fLW Lateral clearance adjustment TRD adjustment Free-flow speed, FFS	nt, fLC	65.0 - - - 65.0	mi/h mi/h mi/h mi/h mi/h
L(OS and Performan	ce Measures	
Flow rate, vp Free-flow speed, FFS Average passenger-car speed Number of lanes, N Density, D		2095 65.0 58.2 4 36.0	pc/h/ln mi/h mi/h pc/mi/ln
Level of service, LOS		E	

Alternative 1: Lengthen the Deceleration Lane at the Exit 36 Diverge Area

- 1. 2025 AM Merge
- 2. 2025 AM Diverge
- 3. 2025 AM Basis Freeway

Fax:

Phone:

E-mail: _____Merge Analysis___ Seth Asante Analyst: Agency/Co.: CTPS Agency/Co.:

Date performed:

Analysis time period:

AM Peak Hour Freeway/Dir of Travel: I-93 SB (I-95 to Montvale Ave) Junction: Merge from I-95 NB to I-93 SB

Jurisdiction: Highway District 4

Analysis Year: 2025 Future Year Description: Low-Cost Improvements to Bottleneck Locations _____Freeway Data_____ Type of analysis Merge Number of lanes in freeway 4 65.0 6050 Free-flow speed on freeway mph Volume on freeway vph On Ramp Data____ Side of freeway Right Number of lanes in ramp 1 Free-flow speed on ramp 30.0 mph 1470 Volume on ramp vph Length of first accel/decel lane 1500 ft Length of second accel/decel lane _____Adjacent Ramp Data (if one exists)_____ Yes 890 Does adjacent ramp exist? Volume on adjacent Ramp vph Position of adjacent Ramp Upstream Type of adjacent Ramp Off 1000 ft Distance to adjacent Ramp _____Conversion to pc/h Under Base Conditions_____ Junction Components Freeway Ramp Adjacent Ramp
6050 1470 890
0.95 0.95 0.95
1592 387 234
4 3 3 3
0 0 0 Level Level Level
% %
mi mi
1.5 1.5 1.5
1.2 1.2 1.2 Volume, V (vph) vph Peak-hour factor, PHF Peak 15-min volume, v15 Trucks and buses Recreational vehicles Terrain type: Grade Length mi Trucks and buses PCE, ET Recreational vehicle PCE, ER

```
6628
                                             1603
                                                        970
Flow rate, vp
                                                                 pcph
                     Estimation of V12 Merge Areas
                L =
                              (Equation 13-6 or 13-7)
                ΕQ
                     0.017 Using Equation 0
                FM
                v = v (P) = 115
                                    pc/h
                 12 F FM
                        Capacity Checks
                                                  LOS F?
                        Actual
                                    Maximum
                        8231
                                    9400
                                                   No
    V
     FO
                        3256 pc/h
                                   (Equation 13-14 or 13-17)
    v or v
     3
          av34
               > 2700 pc/h?
Ιs
    v or v
                                    Yes
     3
         av34
                > 1.5 v /2
Ιs
    v or v
                                    No
          av34
                  12
     3
If yes, v
          = 1228
                                  (Equation 13-15, 13-16, 13-18, or 13-19)
       12A
                    Flow Entering Merge Influence Area
                   Actual Max Desirable
                                                   Violation?
                                4600
                   2831
                                                    No
     12A
              Level of Service Determination (if not F)
Density, D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 17.4 pc/mi/ln
                                     12
Level of service for ramp-freeway junction areas of influence B
                  Speed Estimation
Intermediate speed variable,
                                        M = 0.297
                                        S
Space mean speed in ramp influence area,
                                        S = 58.2
                                                   mph
                                        R
Space mean speed in outer lanes,
                                        S = 56.1
                                                   mph
                                        0
```

S = 56.8

mph

0.980

0.98

0.985

0.98

0.985

0.98

Heavy vehicle adjustment, fHV

Driver population factor, fP

Phone: E-mail: Fax:

_____Diverge Analysis_____

Analyst:	Seth Asante
Agency/Co.:	CTPS
Date performed:	5/20/2015

Date performed: 5/20/2015
Analysis time period: AM Peak Hour

Freeway/Dir of Travel: I-93 SB (I-95 to Montvale Ave)
Junction: Diverge I-93 SB to Montvale

Jurisdiction: Highway District 4
Analysis Year: 2025 Future Year

Description: Low-Cost Improvements to Bottleneck Locations

F	ree	way	Dat	a
		1	_ ~ ~	_

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	65.0	mph
Volume on freeway	6600	vph

Off Ramp Data_____

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	30.0	mph
Volume on ramp	1500	vph
Length of first accel/decel lane	1500	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? Volume on adjacent ramp	Yes 160	vph
Position of adjacent ramp	Downstream	
Type of adjacent ramp	On	
Distance to adjacent ramp	700	ft

______Conversion to pc/h Under Base Conditions_____

Junction Components	Freeway	Ramp	Adjacent Ramp
Volume, V (vph)	6600	1500	160 vph
Peak-hour factor, PHF	0.95	0.95	0.95
Peak 15-min volume, v15	1737	395	42 v
Trucks and buses	4	3	3 %
Recreational vehicles	0	0	0 %
Terrain type:	Level	Level	Level
Grade	0.00 %	0.00 %	0.00 %
Length	0.00 mi	0.00 mi	0.00 mi
Trucks and buses PCE, ET	3.0*	3.0*	3.0*
Recreational vehicle PCE, ER	1.2	1.2	1.2

```
Driver population factor, fP
                                     0.98
                                                 0.98
                                                            0.98
                                     7656
Flow rate, vp
                                                1708
                                                            182
                                                                     pcph
                      Estimation of V12 Diverge Areas
                 L =
                                (Equation 13-12 or 13-13)
                 ΕQ
                        0.436 Using Equation 0
                 FD
                 v = v + (v - v) P = 4301 pc/h
                           F R FD
                  12
                     R
                          Capacity Checks
                                                      LOS F?
                         Actual
                                       Maximum
                         7656
                                       9400
                                                      No
    v = v
     Fi F
                         5948
                                       9400
                                                      No
    v = v - v
     FO
          \mathbf{F}
            R
                         1708
                                       2000
                                                      No
     R
    v or v
                         1677 pc/h (Equation 13-14 or 13-17)
     3
          av34
Ιs
    v or v
               > 2700 pc/h?
                                       No
     3
           av34
                 > 1.5 v /2
Ts
    v or v
                                       No
     3
          av34
                       12
                                    (Equation 13-15, 13-16, 13-18, or 13-19)
If yes, v = 4301
        12A
                     Flow Entering Diverge Influence Area
                                                       Violation?
                     Actual
                                  Max Desirable
                     4301
                                  4400
                                                       No
    v
     12
               Level of Service Determination (if not F)
                      D = 4.252 + 0.0086 \text{ v} - 0.009 \text{ L} = 27.7 \text{ pc/mi/ln}
Density,
                      R
                                         12 D
Level of service for ramp-freeway junction areas of influence C
                 Speed Estimation
                                           D = 0.647
Intermediate speed variable,
                                           S
Space mean speed in ramp influence area,
                                           S = 50.1
                                                       mph
                                           R
Space mean speed in outer lanes,
                                           S = 68.7
                                                       mph
                                           0
```

S = 56.9

mph

0.926

0.943

0.943

Heavy vehicle adjustment, fHV

Phone: E-mail:		Fax:	
	Operational Ana	lvsis	
		-,0-0	
Analyst: Agency or Company: Date Performed: Analysis Time Period: Freeway/Direction: From/To: Jurisdiction: Analysis Year: Description: Low-Cost	I-93 Southbound I-95 to Montvale Highway District 2025	4	ions
	Flow Inputs and	Adjustments	
Volume, V Peak-hour factor, PHF Peak 15-min volume, v15	5	7560 0.95 1989	veh/h v
Trucks and buses		4	9
Recreational vehicles		0	ଚ
Terrain type:		Level	
Grade		-	°6
Segment length Trucks and buses PCE, F	r m	<u>-</u> 1.5	mi
Recreational vehicle PO		1.2	
Heavy vehicle adjustmen	-	0.980	
Driver population factor		0.98	
Flow rate, vp	-	2071	pc/h/ln
	Speed Inputs an	d Adjustments	
Lane width		_	ft
Right-side lateral clea	arance	-	ft
Total ramp density, TRI)	-	ramps/mi
Number of lanes, N		4	
Free-flow speed:		Measured	
FFS or BFFS	C	65.0	mi/h
Lane width adjustment,		_	mi/h
Lateral clearance adjust TRD adjustment	stment, ill	_	mi/h mi/h
Free-flow speed, FFS		65.0	mi/h
-	LOS and Perform	ance Measures	
			
Flow rate, vp		2071	pc/h/ln
Free-flow speed, FFS	d C	65.0	mi/h
Average passenger-car s	speed, S	58.6	mi/h
Number of lanes, N Density, D		4 35.3	pc/mi/ln
Level of service LOS		50.5 F	PC/ m1/ 111

Ε

Level of service, LOS

Alternative 1: Lengthen the Deceleration Lane at the Exit 36 Diverge Area

- 1. 2025 PM Merge
- 2. 2025 PM Diverge
- 3. 2025 PM Basis Freeway

Phone: Fax: E-mail: _____Merge Analysis___ Seth Asante Analyst: Agency/Co.: CTPS Agency/Co.:

Date performed: 5/20/2015

Analysis time period: PM Peak Hour Freeway/Dir of Travel: I-93 SB (I-95 to Montvale Ave) Junction: Merge from I-95 NB to I-93 SB

Jurisdiction: Highway District 4

Analysis Year: 2025 Future Year Description: Low-Cost Improvements to Bottleneck Locations ______Freeway Data_____ Type of analysis Merge Number of lanes in freeway 4 65.0 6300 Free-flow speed on freeway mph Volume on freeway vph _____On Ramp Data_____ Side of freeway Right Number of lanes in ramp 1 Free-flow speed on ramp 30.0 mph Volume on ramp 1700 vph Length of first accel/decel lane 1500 ft Length of second accel/decel lane _____Adjacent Ramp Data (if one exists)_____ Yes 890 Does adjacent ramp exist? Volume on adjacent Ramp vph Position of adjacent Ramp Upstream Type of adjacent Ramp Off 1000 ft Distance to adjacent Ramp _____Conversion to pc/h Under Base Conditions_____ Freeway Ramp Junction Components Adjacent Ramp
6300 1700 890
0.96 0.96 0.96
1641 443 232
4 3 3
0 0 0 0
Level Level Level
% mi mi
1.5 1.5 1.5
1.2 1.2 1.2 Volume, V (vph) vph Peak-hour factor, PHF Peak 15-min volume, v15 Trucks and buses Recreational vehicles Terrain type: Grade

mi

Length

Trucks and buses PCE, ET Recreational vehicle PCE, ER

```
6830
                                              1834
                                                         960
Flow rate, vp
                                                                 pcph
                     Estimation of V12 Merge Areas
                L =
                               (Equation 13-6 or 13-7)
                ΕQ
                      -0.011 Using Equation 0
                FM
                v = v (P) = -77
                                    pc/h
                 12 F FM
                        Capacity Checks
                                                  LOS F?
                        Actual
                                     Maximum
                        8664
                                     9400
                                                   No
    V
     FO
                        3453 pc/h
                                   (Equation 13-14 or 13-17)
    v or v
     3
          av34
                > 2700 pc/h?
Ιs
    v or v
                                    Yes
     3
         av34
                > 1.5 v /2
Ιs
    v or v
                                    Yes
          av34
                     12
     3
If yes, v
          = 2732
                                  (Equation 13-15, 13-16, 13-18, or 13-19)
       12A
                    Flow Entering Merge Influence Area
                   Actual Max Desirable
                                                    Violation?
                   4566
                                4600
                                                    No
     12A
              Level of Service Determination (if not F)
Density, D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 30.8 pc/mi/ln
                                     12
Level of service for ramp-freeway junction areas of influence D
                  Speed Estimation
Intermediate speed variable,
                                        M = 0.606
                                         S
Space mean speed in ramp influence area,
                                        S = 51.1
                                                    mph
                                         R
Space mean speed in outer lanes,
                                        S = 59.4
                                                    mph
                                         0
Space mean speed for all vehicles,
```

S = 54.7

mph

0.980

0.98

0.985

0.98

0.985

0.98

Heavy vehicle adjustment, fHV

Driver population factor, fP

Phone: E-mail:

Fax:

_____Diverge Analysis_____

Analyst:	Seth Asante
Agency/Co.:	CTPS
Date performed:	5/20/2015
Analysis time period:	PM Peak Hour
Freeway/Dir of Travel:	I-93 SB (I-95

Freeway/Dir of Travel: I-93 SB (I-95 to Montvale Ave)
Junction: Diverge I-93 SB to Montvale Av

Jurisdiction: Highway District 4
Analysis Year: 2025 Future Year

Description: Low-Cost Improvements to Bottleneck Locations

		_Freeway	Data
TT	of onolysis		Dirrommo

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	65.0	mph
Volume on freeway	8000	vph

Off	Ramp	Data
~		

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	30.0	mph
Volume on ramp	1050	vph
Length of first accel/decel lane	1500	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)_____

Does adjacent ramp exist? Volume on adjacent ramp	Yes 370	vph
Position of adjacent ramp	Downstream	-
Type of adjacent ramp	On	
Distance to adjacent ramp	700	ft

______Conversion to pc/h Under Base Conditions_____

Junction Components	Freeway	Ramp	Adjacent
			Ramp
Volume, V (vph)	8000	1050	370 vph
Peak-hour factor, PHF	0.96	0.96	0.96
Peak 15-min volume, v15	2083	273	96 v
Trucks and buses	4	4	3 %
Recreational vehicles	0	0	0 %
Terrain type:	Level	Level	Level
Grade	0.00 %	0.00 %	0.00 %
Length	0.00 mi	0.00 mi	0.00 mi
Trucks and buses PCE, ET	3.0*	3.0*	3.0*
Recreational vehicle PCE, ER	1.2	1.2	1.2

```
Driver population factor, fP
                                     0.98
                                                0.98
                                                            0.98
                                     9184
Flow rate, vp
                                                1205
                                                            417
                                                                     pcph
                      Estimation of V12 Diverge Areas
                 L =
                                (Equation 13-12 or 13-13)
                 ΕQ
                        0.436 Using Equation 0
                 FD
                 v = v + (v - v) P = 4684 pc/h
                           F R FD
                  12
                     R
                         Capacity Checks
                                                     LOS F?
                         Actual
                                       Maximum
                         9184
                                       9400
                                                      No
    v = v
     Fi F
                         7979
                                       9400
                                                      No
    v = v - v
     FO
          \mathbf{F}
            R
                         1205
                                       2000
                                                      No
     R
    v or v
                         2250 pc/h (Equation 13-14 or 13-17)
     3 av34
Ιs
    v or v
               > 2700 pc/h?
                                      No
     3
          av34
                > 1.5 v /2
Ts
    v or v
                                      No
     3
          av34
                       12
If yes, v = 4684
                                    (Equation 13-15, 13-16, 13-18, or 13-19)
        12A
                     Flow Entering Diverge Influence Area
                     Actual
                                  Max Desirable
                                                       Violation?
                     4684
                                  4400
                                                       Yes
    v
     12
               Level of Service Determination (if not F)
                     D = 4.252 + 0.0086 \text{ v} - 0.009 \text{ L} = 31.0 \text{ pc/mi/ln}
Density,
                      R
                                        12 D
Level of service for ramp-freeway junction areas of influence D
                Speed Estimation
                                          D = 0.601
Intermediate speed variable,
                                           S
Space mean speed in ramp influence area,
                                          S = 51.2
                                                       mph
                                           R
Space mean speed in outer lanes,
                                          S = 66.4
                                                       mph
                                           0
```

S = 57.7

mph

0.926

0.926

0.943

Heavy vehicle adjustment, fHV

Phone: Fax: E-mail: _____Operational Analysis_____ Analyst: Seth Agency or Company: CTPS
Date Performed: 8/31/2015
Analysis Time Period: PM Peak Hour Jurisdiction: Highway District 4
Analysis Year: 2025
Description: Description: Low-Cost Improvements to Bottleneck Locations _____Flow Inputs and Adjustments_____ veh/h Volume, V 8050 Peak-hour factor, PHF 0.95 Peak 15-min volume, v15 2118 V 4 0 Trucks and buses Recreational vehicles Level Terrain type: Grade Segment length тi Trucks and buses PCE, ET 1.5 Recreational vehicle PCE, ER 1.2 Heavy vehicle adjustment, fHV 0.980 Driver population factor, fp 0.98 Flow rate, vp 2205 pc/h/ln Speed Inputs and Adjustments Lane width ft Right-side lateral clearance ft Total ramp density, TRD ramps/mi Number of lanes, N 4 Free-flow speed: Measured FFS or BFFS 65.0 mi/h Lane width adjustment, fLW mi/h Lateral clearance adjustment, fLC mi/h TRD adjustment mi/h Free-flow speed, FFS 65.0 mi/h LOS and Performance Measures_____ Flow rate, vp 2205 pc/h/ln Free-flow speed, FFS 65.0 mi/h Average passenger-car speed, S 55.8 mi/h Number of lanes, N 4 39.5 Density, D pc/mi/ln

Ε

Level of service, LOS

Alternative 2: Create an Auxiliary Lane for Merging and Diverging Traffic

- 1. 2025 AM Merge
- 2. 2025 AM Diverge
- 3. 2025 AM Basis Freeway

Phone: Fax: E-mail: _____Merge Analysis___ Seth Asante Analyst: Agency/Co.: CTPS Agency/Co.:

Date performed: 5/20/2015

Analysis time period: AM Peak Hour Freeway/Dir of Travel: I-93 SB (I-95 to Montvale Ave) Junction: Merge from I-95 NB to I-93 SB

Jurisdiction: Highway District 4

Analysis Year: 2025 Future Year Alternative 2 Description: Low-Cost Improvements to Bottleneck Locations _____Freeway Data_____ Type of analysis Merge Number of lanes in freeway 4 65.0 6050 Free-flow speed on freeway mph Volume on freeway vph On Ramp Data____ Side of freeway Right Number of lanes in ramp 1 Free-flow speed on ramp 30.0 mph 1470 Volume on ramp vph Length of first accel/decel lane 1500 ft Length of second accel/decel lane _____Adjacent Ramp Data (if one exists)_____ Yes 890 Does adjacent ramp exist? Volume on adjacent Ramp vph Position of adjacent Ramp Upstream Type of adjacent Ramp Off 1000 ft Distance to adjacent Ramp _____Conversion to pc/h Under Base Conditions_____ Junction Components Freeway Ramp Adjacent Ramp
6050 1470 890
0.95 0.95 0.95
1592 387 234
4 3 3 3
0 0 0 0
Level Level Level
% %
mi mi
1.5 1.5 1.5
1.2 1.2 1.2 Volume, V (vph) vph Peak-hour factor, PHF Peak 15-min volume, v15 Trucks and buses Recreational vehicles Terrain type: Grade

mi

Length

Trucks and buses PCE, ET Recreational vehicle PCE, ER

```
6628
                                             1603
                                                        970
Flow rate, vp
                                                                 pcph
                     Estimation of V12 Merge Areas
                L =
                              (Equation 13-6 or 13-7)
                ΕQ
                     0.017 Using Equation 0
                FM
                v = v (P) = 115
                                    pc/h
                 12 F FM
                        Capacity Checks
                                                  LOS F?
                        Actual
                                    Maximum
                        8231
                                    9400
                                                   No
    V
     FO
                        3256 pc/h
                                   (Equation 13-14 or 13-17)
    v or v
     3
          av34
               > 2700 pc/h?
Ιs
    v or v
                                    Yes
     3
         av34
                > 1.5 v /2
Ιs
    v or v
                                    No
          av34
                  12
     3
If yes, v
          = 1228
                                  (Equation 13-15, 13-16, 13-18, or 13-19)
       12A
                    Flow Entering Merge Influence Area
                   Actual Max Desirable
                                                   Violation?
                                4600
                   2831
                                                    No
     12A
              Level of Service Determination (if not F)
Density, D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 17.4 pc/mi/ln
                                     12
Level of service for ramp-freeway junction areas of influence B
                  Speed Estimation
Intermediate speed variable,
                                        M = 0.297
                                        S
Space mean speed in ramp influence area,
                                        S = 58.2
                                                   mph
                                        R
Space mean speed in outer lanes,
                                        S = 56.1
                                                   mph
                                        0
```

S = 56.8

mph

0.980

0.98

0.985

0.98

0.985

0.98

Heavy vehicle adjustment, fHV

Driver population factor, fP

Phone: E-mail:

Analyst:

Fax:

_____Diverge Analysis_____

Seth Asante

Agency/Co.: CTPS Date performed: 5/20/2015 Analysis time period: AM Peak Hour Freeway/Dir of Travel: I-93 SB (I-95 to Montvale Ave) Junction: Diverge I-93 SB to Montvale Jurisdiction: Highway District 4 Analysis Year: 2025 Future Year Alt 2			
Description: Low-Cost Imp	provements to Bottl	eneck Location	S
	Freeway Da	ıta	
Type of analysis Diverge Number of lanes in freeway 4 Free-flow speed on freeway 65.0 Volume on freeway 6600		4 65.0	mph vph
	Off Ramp Da	ıta	
Side of freeway		Right	
Number of lanes in ramp		1	
Free-Flow speed on ramp		30.0	mph
Volume on ramp 1500			vph
Length of first accel/decel lane 1500			ft
Length of second accel/ded	cel lane		ft
Adjacent Ramp Data (if one exists)			

_____Conversion to pc/h Under Base Conditions_____

Junction Components	Freeway	Ramp	Adjacent Ramp
Volume, V (vph)	6600	1500	160 vph
Peak-hour factor, PHF	0.95	0.95	0.95
Peak 15-min volume, v15	1737	395	42 v
Trucks and buses	4	3	3 %
Recreational vehicles	0	0	0 %
Terrain type:	Level	Level	Level
Grade	0.00 응	0.00 응	0.00 %
Length	0.00 mi	0.00 mi	0.00 mi
Trucks and buses PCE, ET	3.0*	3.0*	3.0*
Recreational vehicle PCE, ER	1.2	1.2	1.2

```
Driver population factor, fP
                                     0.98
                                                 0.98
                                                            0.98
                                     7656
Flow rate, vp
                                                1708
                                                            182
                                                                     pcph
                      Estimation of V12 Diverge Areas
                 L =
                                (Equation 13-12 or 13-13)
                 ΕQ
                        0.436 Using Equation 0
                 FD
                 v = v + (v - v) P = 4301 pc/h
                           F R FD
                  12
                     R
                          Capacity Checks
                                                      LOS F?
                         Actual
                                       Maximum
                         7656
                                       9400
                                                      No
    v = v
     Fi F
                         5948
                                       9400
                                                      No
    v = v - v
     FO
          \mathbf{F}
            R
                         1708
                                       2000
                                                      No
     R
    v or v
                         1677 pc/h (Equation 13-14 or 13-17)
     3
          av34
Ιs
    v or v
               > 2700 pc/h?
                                       No
     3
           av34
                 > 1.5 v /2
Ts
    v or v
                                       No
     3
          av34
                       12
                                    (Equation 13-15, 13-16, 13-18, or 13-19)
If yes, v = 4301
        12A
                     Flow Entering Diverge Influence Area
                                                       Violation?
                     Actual
                                  Max Desirable
                     4301
                                  4400
                                                       No
    v
     12
               Level of Service Determination (if not F)
                      D = 4.252 + 0.0086 \text{ v} - 0.009 \text{ L} = 27.7 \text{ pc/mi/ln}
Density,
                      R
                                         12 D
Level of service for ramp-freeway junction areas of influence C
                 Speed Estimation
                                           D = 0.647
Intermediate speed variable,
                                           S
Space mean speed in ramp influence area,
                                           S = 50.1
                                                       mph
                                           R
Space mean speed in outer lanes,
                                           S = 68.7
                                                       mph
                                           0
```

S = 56.9

mph

0.926

0.943

0.943

Heavy vehicle adjustment, fHV

Phone: E-mail:		Fax:	
	Operational Ana	alysis	
Analyst: Agency or Company: Date Performed: Analysis Time Period: Freeway/Direction: From/To: Jurisdiction: Analysis Year: Description: Low-cost	Seth CTPS 8/6/2015 AM Peak I-93 Southbound Auxiliary Lane Highway District 2025 Future Year Improvements to E	:	93 SB
	Flow Inputs and	l Adjustments	
Volume, V Peak-hour factor, PHF Peak 15-min volume, v15		7550 0.96 1966	veh/h
Trucks and buses)	4	V %
Recreational vehicles		0	8
Terrain type: Grade		Level	90
Segment length		_	mi
Trucks and buses PCE, F Recreational vehicle PC Heavy vehicle adjustment Driver population factor Flow rate, vp	CE, ER nt, fHV	1.5 1.2 0.980 1.00 1604	pc/h/ln
	Speed Inputs ar	nd Adiustments	
Lane width Right-side lateral clea	ranco	_	ft ft
Total ramp density, TRI		<u>-</u>	ramps/mi
Number of lanes, N		5	
Free-flow speed:		Measured	
FFS or BFFS	CITI	65.0	mi/h
Lane width adjustment, Lateral clearance adjus		-	mi/h mi/h
TRD adjustment	cinenc, inc	_	mi/h
Free-flow speed, FFS		65.0	mi/h
	LOS and Perform	nance Measures	
Flow rate wa		1604	pc/h/ln
Flow rate, vp Free-flow speed, FFS		65.0	mi/h
Average passenger-car s	speed, S	64.4	mi/h
Number of lanes, N	± ,	5	·
Density, D		24.9	pc/mi/ln
Level of service LOS		C	

С

Level of service, LOS

Alternative 2: Create an Auxiliary Lane for Merging and Diverging Traffic

- 1. 2025 PM Merge
- 2. 2025 PM Diverge
- 3. 2025 PM Basis Freeway

Phone: Fax: E-mail: Merge Analysis Analyst: Agency/Co.: CTPS Agency/Co.: CTPS
Date performed: 5/20/2015
Analysis time period: PM Peak Hour Freeway/Dir of Travel: I-93 SB (I-95 to Montvale Ave) Junction: Merge from I-95 NB to I-93 SB Jurisdiction: Highway District 4
Analysis Year: 2025 Future Year Alternative 2 Description: Low-Cost Improvements to Bottleneck Locations _____Freeway Data_____ Type of analysis Merge Number of lanes in freeway 4 65.0 6300 Free-flow speed on freeway mph Volume on freeway vph On Ramp Data____ Side of freeway Right Number of lanes in ramp 1 Free-flow speed on ramp 30.0 mph Volume on ramp 1700 vph Length of first accel/decel lane 1500 ft Length of second accel/decel lane _____Adjacent Ramp Data (if one exists)_____ Yes 890 Does adjacent ramp exist? Volume on adjacent Ramp vph Position of adjacent Ramp Upstream Type of adjacent Ramp Off 1000 ft Distance to adjacent Ramp _____Conversion to pc/h Under Base Conditions_____ Freeway Ramp Junction Components Adjacent Ramp
6300 1700 890
0.96 0.96 0.96
1641 443 232
4 3 3
0 0 0 0
Level Level Level
% %
mi mi
1.5 1.5 1.5
1.2 1.2 1.2 Volume, V (vph) vph Peak-hour factor, PHF Peak 15-min volume, v15 Trucks and buses Recreational vehicles Terrain type: Grade Length mi Trucks and buses PCE, ET

Recreational vehicle PCE, ER

```
6830
                                              1834
                                                         960
Flow rate, vp
                                                                 pcph
                     Estimation of V12 Merge Areas
                L =
                               (Equation 13-6 or 13-7)
                ΕQ
                      -0.011 Using Equation 0
                FM
                v = v (P) = -77
                                    pc/h
                 12 F FM
                        Capacity Checks
                                                  LOS F?
                        Actual
                                     Maximum
                        8664
                                     9400
                                                   No
    V
     FO
                        3453 pc/h
                                   (Equation 13-14 or 13-17)
    v or v
     3
          av34
                > 2700 pc/h?
Ιs
    v or v
                                    Yes
     3
         av34
                > 1.5 v /2
Ιs
    v or v
                                    Yes
          av34
                     12
     3
If yes, v
          = 2732
                                  (Equation 13-15, 13-16, 13-18, or 13-19)
       12A
                    Flow Entering Merge Influence Area
                   Actual Max Desirable
                                                    Violation?
                   4566
                                4600
                                                    No
     12A
              Level of Service Determination (if not F)
Density, D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 30.8 pc/mi/ln
                                     12
Level of service for ramp-freeway junction areas of influence D
                  Speed Estimation
Intermediate speed variable,
                                        M = 0.606
                                         S
Space mean speed in ramp influence area,
                                        S = 51.1
                                                    mph
                                         R
Space mean speed in outer lanes,
                                        S = 59.4
                                                    mph
                                         0
Space mean speed for all vehicles,
```

S = 54.7

mph

0.980

0.98

0.985

0.98

0.985

0.98

Heavy vehicle adjustment, fHV

Driver population factor, fP

Phone: E-mail: Fax:

_____Diverge Analysis_____

Analyst: Seth Asante Agency/Co.: CTPS Date performed: 5/20/2015
Analysis time period: PM Peak Hour

Freeway/Dir of Travel: I-93 SB (I-95 to Montvale Ave)

Junction: Diverge I-93 SB to Montvale Av Jurisdiction: Highway District 4 Analysis Year: 2025 Future Year

Description: Low-Cost Improvements to Bottleneck Locations

F	'r	е	е	W	ау	Da	Э.	ti	а
 F.	r	е	е	W	ау	Da	₹.	C	a

Type of analysis	Diverge	
Number of lanes in freeway	4	
Free-flow speed on freeway	65.0	mph
Volume on freeway	8000	vph

Off Ramp Data_____

Side of freeway	Right	
Number of lanes in ramp	1	
Free-Flow speed on ramp	30.0	mph
Volume on ramp	1050	vph
Length of first accel/decel lane	1500	ft
Length of second accel/decel lane		ft

Adjacent Ramp Data (if one exists)

Does adjacent ramp exist? Yes 370 Volume on adjacent ramp vph Position of adjacent ramp Downstream Type of adjacent ramp On Distance to adjacent ramp 700 ft

_____Conversion to pc/h Under Base Conditions_____

Junction Components	Freeway	Ramp	Adjacent Ramp
Volume, V (vph)	8000	1050	370 vph
Peak-hour factor, PHF	0.96	0.96	0.96
Peak 15-min volume, v15	2083	273	96 v
Trucks and buses	4	3	3 %
Recreational vehicles	0	0	0 %
Terrain type:	Level	Level	Level
Grade	0.00 %	0.00 %	0.00 %
Length	0.00 mi	0.00 mi	0.00 mi
Trucks and buses PCE, ET	3.0*	3.0*	3.0*
Recreational vehicle PCE, ER	1.2	1.2	1.2

```
Driver population factor, fP
                                     0.98
                                                0.98
                                                            0.98
                                     9184
Flow rate, vp
                                                1183
                                                            417
                                                                     pcph
                      Estimation of V12 Diverge Areas
                 L =
                                (Equation 13-12 or 13-13)
                 ΕQ
                        0.436 Using Equation 0
                 FD
                 v = v + (v - v) P = 4671 pc/h
                           F R FD
                  12
                     R
                          Capacity Checks
                                                     LOS F?
                         Actual
                                       Maximum
                         9184
                                       9400
                                                      No
    v = v
     Fi F
                         8001
                                       9400
                                                      No
    v = v - v
     FO
          \mathbf{F}
            R
                         1183
                                       2000
                                                      No
     R
    v or v
                         2256 pc/h (Equation 13-14 or 13-17)
     3
         av34
Ιs
    v or v
               > 2700 pc/h?
                                      No
     3
          av34
                > 1.5 v /2
Ts
    v or v
                                       No
     3
          av34
                       12
If yes, v = 4671
                                    (Equation 13-15, 13-16, 13-18, or 13-19)
        12A
                     Flow Entering Diverge Influence Area
                     Actual
                                  Max Desirable
                                                       Violation?
                     4671
                                  4400
                                                       Yes
    v
     12
               Level of Service Determination (if not F)
                      D = 4.252 + 0.0086 \text{ v} - 0.009 \text{ L} = 30.9 \text{ pc/mi/ln}
Density,
                      R
                                         12 D
Level of service for ramp-freeway junction areas of influence D
                 Speed Estimation
                                           D = 0.599
Intermediate speed variable,
                                           S
Space mean speed in ramp influence area,
                                          S = 51.2
                                                       mph
                                           R
Space mean speed in outer lanes,
                                           S = 66.4
                                                       mph
                                           0
```

S = 57.7

mph

0.926

0.943

0.943

Heavy vehicle adjustment, fHV

Fax:					
erational Analysis					
S /2015 Peak 3 Southbound iliary Lane hway District 4 5 Future Year	E-93 SB				
ow Inputs and Adjustments					
8050 0.96 2096 4 0 Level	veh/h v %				
HV 0.980	% mi pc/h/ln				
eed Inputs and Adjustments_					
- 5 Measured 65.0 -	<pre>ft ft ramps/mi mi/h mi/h mi/h mi/h mi/h mi/h mi/h</pre>				
LOS and Performance Measures					
1711 65.0 63.6 5 26.9	pc/h/ln mi/h mi/h pc/mi/ln				
	cerational Analysis				

D

Level of service, LOS

LOCATION 2

I-95 Southbound at the I-90 Interchange in Weston

2015 Existing Conditions

- 1. 2015 AM Diverge to I-90
- 2. 2015 AM Diverge to Route 30
- 3. 2015 AM Merge onto Route 30
- 4. 2015 AM Merge onto I-90

Phone: Fax: E-mail: _____Diverge Analysis_____ Seth Analyst: Agency/Co.: CTPS Agency/Co.:

Date performed:

Analysis time period:

AM Peak Hour Freeway/Dir of Travel: I-95 Southbound Junction: Diverge to Mass Pike Jurisdiction: Highway District 6 Analysis Year: 2015 Existing Description: Low-Cost Improvements to Bottleneck Locations _____Freeway Data_____ Type of analysis Diverge Number of lanes in freeway 4 55.0 7100 Free-flow speed on freeway mph Volume on freeway vph Off Ramp Data____ Side of freeway Right Number of lanes in ramp 1 Free-Flow speed on ramp 25.0 mph Volume on ramp 1750 vph Length of first accel/decel lane 350 ft Length of second accel/decel lane _____Adjacent Ramp Data (if one exists)__ Yes 900 Does adjacent ramp exist? Volume on adjacent ramp vph Position of adjacent ramp Downstream Off Type of adjacent ramp 1400 ft Distance to adjacent ramp _____Conversion to pc/h Under Base Conditions_____ Junction Components Freeway Ramp Adjacent Ramp
7100 1750 900 V
0.95 0.95 0.95
1868 461 237 V
4 3 3 3 8
0 0 0 0 0 8
Level Level Level
0.00 % 0.00 % 0.00 %
0.00 mi 0.00 mi 0.00 mi
1.5 1.5 1.5
1.2 1.2 1.2 Volume, V (vph) vph Peak-hour factor, PHF Peak 15-min volume, v15 Trucks and buses Recreational vehicles Terrain type: Grade

Length

Trucks and buses PCE, ET

Recreational vehicle PCE, ER

```
Driver population factor, fP
                                     0.98
                                                 0.98
                                                            0.98
                                     7779
Flow rate, vp
                                                1908
                                                            981
                                                                     pcph
                      Estimation of V12 Diverge Areas
                 L =
                                (Equation 13-12 or 13-13)
                 ΕQ
                        0.436 Using Equation 0
                 FD
                 v = v + (v - v) P = 4468 pc/h
                           F R FD
                  12
                     R
                          Capacity Checks
                                                      LOS F?
                         Actual
                                       Maximum
                          7779
                                       9000
                                                      No
    v = v
     Fi F
                          5871
                                       9000
                                                      No
    v = v - v
     FO
          \mathbf{F}
            R
                         1908
                                       1900
                                                      Yes
     R
    v or v
                         1655 pc/h (Equation 13-14 or 13-17)
     3
          av34
Ιs
    v or v
               > 2700 pc/h?
                                       No
     3
           av34
                 > 1.5 v /2
Ts
    v or v
                                       No
     3
          av34
                       12
If yes, v = 4468
                                    (Equation 13-15, 13-16, 13-18, or 13-19)
        12A
                     Flow Entering Diverge Influence Area
                     Actual
                                  Max Desirable
                                                       Violation?
                     4468
                                  4400
                                                       Yes
    v
     12
               Level of Service Determination (if not F)
                      D = 4.252 + 0.0086 \text{ v} - 0.009 \text{ L} = 39.5 \text{ pc/mi/ln}
Density,
                      R
                                         12 D
Level of service for ramp-freeway junction areas of influence F
                 Speed Estimation
                                           D = 0.730
Intermediate speed variable,
                                           S
Space mean speed in ramp influence area,
                                           S = 45.5
                                                       mph
                                           R
Space mean speed in outer lanes,
                                           S = 57.8
                                                       mph
                                           0
```

S = 50.0

mph

0.980

0.985

0.985

Heavy vehicle adjustment, fHV

Space mean speed for all vehicles,

Phone: Fax: E-mail: _____Diverge Analysis Seth Analyst: Agency/Co.: CTPS Agency/Co.: CTPS
Date performed: 6/2/2015
Analysis time period: AM Peak Freeway/Dir of Travel: I-95 Southbound Junction: Diverge to Route 30
Jurisdiction: Highway District 6
Analysis Year: 2015 Existing Description: Low-cost Improvements to Bottleneck Locations _____Freeway Data_____ Type of analysis Diverge Number of lanes in freeway 4 55.0 5350 Free-flow speed on freeway mph Volume on freeway vph Off Ramp Data____ Side of freeway Right Number of lanes in ramp 1 Free-Flow speed on ramp mph 25.0 Volume on ramp 900 vph Length of first accel/decel lane 500 ft Length of second accel/decel lane _____Adjacent Ramp Data (if one exists)__ Yes 550 Does adjacent ramp exist? Volume on adjacent ramp vph Position of adjacent ramp Downstream Type of adjacent ramp On ft Distance to adjacent ramp 400 _____Conversion to pc/h Under Base Conditions_____ Freeway Ramp Junction Components Adjacent Ramp

5350 900 550 V

0.93 0.92 0.92

1438 245 149 V

4 4 4 4 8

0 0 0 0 0 8

Level Level Level

0.00 % 0.00 % 0.00 %

0.00 mi 0.00 mi 0.00 mi

1.5 1.5 1.5

1.2 1.2 1.2 Volume, V (vph) vph Peak-hour factor, PHF Peak 15-min volume, v15 Trucks and buses Recreational vehicles Terrain type: Grade

Length

Trucks and buses PCE, ET Recreational vehicle PCE, ER

```
5987
Flow rate, vp
                                                1018
                                                            622
                                                                     pcph
                      Estimation of V12 Diverge Areas
                 L =
                                (Equation 13-12 or 13-13)
                 ΕQ
                        0.436 Using Equation 0
                 FD
                 v = v + (v - v) P = 3184 pc/h
                           F R FD
                  12
                     R
                         Capacity Checks
                                                     LOS F?
                         Actual
                                       Maximum
                         5987
                                       9000
                                                      No
    v = v
     Fi F
                         4969
                                       9000
                                                      No
    v = v - v
     FO
          \mathbf{F}
            R
                         1018
                                       1900
                                                      No
     R
    v or v
                         1401 pc/h (Equation 13-14 or 13-17)
     3
         av34
Ιs
    v or v
               > 2700 pc/h?
                                      No
     3
           av34
                > 1.5 v /2
Ts
    v or v
                                       No
     3
          av34
                       12
                                    (Equation 13-15, 13-16, 13-18, or 13-19)
If yes, v = 3184
        12A
                     Flow Entering Diverge Influence Area
                                                       Violation?
                     Actual
                                  Max Desirable
                     3184
                                  4400
                                                       No
    v
     12
               Level of Service Determination (if not F)
                     D = 4.252 + 0.0086 \text{ v} - 0.009 \text{ L} = 27.1 \text{ pc/mi/ln}
Density,
                      R
                                        12 D
Level of service for ramp-freeway junction areas of influence C
                Speed Estimation
                                          D = 0.650
Intermediate speed variable,
                                           S
Space mean speed in ramp influence area,
                                          S = 46.6
                                                       mph
                                           R
Space mean speed in outer lanes,
                                          S = 58.8
                                                       mph
                                           0
Space mean speed for all vehicles,
                                          S = 51.6
                                                       mph
```

0.980

0.98

0.980

0.98

0.980

0.98

Heavy vehicle adjustment, fHV

Phone: E-mail:	Fax:					
	ge Analysis					
Herv	ge Anarysis					
Analyst: Seth						
Agency/Co.: CTPS						
Date performed: 6/2/2015						
Analysis time period: AM Peak						
	Freeway/Dir of Travel: I-95 SB					
Junction: Merge from Ro Jurisdiction: Highway Dist:						
Analysis Year: 1918 Existing						
Description: Low-cost Improvements	=	k Locatio	ns			
Topolity Ion Cope implevements						
Fre	eeway Data					
Type of analysis	Merge	Э				
Number of lanes in freeway	4					
Free-flow speed on freeway	55.0		mph			
Volume on freeway	4450		vph			
On	Ramp Data					
Side of freeway	Right	_				
Number of lanes in ramp	1	-				
Free-flow speed on ramp	25.0		mph			
Volume on ramp	550		vph			
Length of first accel/decel lane	450		ft			
Length of second accel/decel lane			ft			
Adjacent Ran	mp Data (if o	one exist	s)			
Does adjacent ramp exist?	Yes					
Volume on adjacent Ramp	1850		vph			
Position of adjacent Ramp		stream	1			
Type of adjacent Ramp	On					
Distance to adjacent Ramp	800		ft			
Conversion to pc	/h Under Base	e Conditi	ons			
Junction Components	Freeway	Ramp		Adjacer	n†	
Januaran Jampanenes	rrccway	Namp		Ramp		
Volume, V (vph)	4450	550		1850	vph	
Peak-hour factor, PHF	0.94	0.94		0.89	_	
Peak 15-min volume, v15	1184	146		520	V	
Trucks and buses	4	4		4	9	
Recreational vehicles	0	0		0	%	
Terrain type:	Level	Level	0	Level	0	
Grade	8	<u>.</u>	앙 !		8	
Length	m:		mi	1 5	mi	
Trucks and buses PCE, ET	1.5 1.2	1.5 1.2		1.5 1.2		
Recreational vehicle PCE, ER	⊥ • ∠	⊥ • ∠		1.2		

```
4927
                                             609
                                                        2163
Flow rate, vp
                                                                 pcph
                     Estimation of V12 Merge Areas
                L =
                              (Equation 13-6 or 13-7)
                ΕQ
                     0.142 Using Equation 0
                FM
                v = v (P) = 698
                                    pc/h
                 12 F FM
                        Capacity Checks
                                                  LOS F?
                        Actual
                                    Maximum
                        5536
                                    9000
                                                  No
    V
     FO
                        2114 pc/h
                                   (Equation 13-14 or 13-17)
    v or v
          av34
     3
               > 2700 pc/h?
Ιs
    v or v
                                    No
     3
         av34
                > 1.5 v /2
Ιs
    v or v
                                    No
          av34
                  12
     3
If yes, v
          = 698
                                  (Equation 13-15, 13-16, 13-18, or 13-19)
       12A
                    Flow Entering Merge Influence Area
                   Actual Max Desirable
                                                   Violation?
                   1307
                                4600
                                                    No
     R12
              Level of Service Determination (if not F)
Density, D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 12.6 pc/mi/ln
                                     12
Level of service for ramp-freeway junction areas of influence B
                  Speed Estimation
Intermediate speed variable,
                                        M = 0.313
                                        S
Space mean speed in ramp influence area,
                                        S = 50.9
                                                   mph
                                        R
                                        S = 49.2
Space mean speed in outer lanes,
                                                   mph
                                        0
Space mean speed for all vehicles,
                                       S = 49.6
                                                   mph
```

0.980

0.98

0.980

0.98

0.980

0.98

Heavy vehicle adjustment, fHV

Fax:

Phone:

E-mail:	rax.				
Merge	e Analysis				
Analyst: Agency/Co.: Date performed: Analysis time period: Freeway/Dir of Travel: Junction: Jurisdiction: Analysis Year: Description: Low-cost Improvements to	ss Pike Lct 6	Location	S		
Free	eway Data				
Type of analysis Number of lanes in freeway Free-flow speed on freeway Volume on freeway	Merge 4 55.0 5000		mph vph		
On F	Ramp Data				
Side of freeway Number of lanes in ramp Free-flow speed on ramp Volume on ramp Length of first accel/decel lane Length of second accel/decel lane	Right 1 25.0 1850 400		mph vph ft ft		
Adjacent Ramp	Data (if or	ne exists)		
Does adjacent ramp exist? Volume on adjacent Ramp Position of adjacent Ramp Type of adjacent Ramp Distance to adjacent Ramp	Yes 550 Upstre On 800	eam	vph ft		
Conversion to pc/h	n Under Base	Conditio	ns		
Junction Components	Freeway	Ramp		Adjacent Ramp	
Volume, V (vph) Peak-hour factor, PHF Peak 15-min volume, v15 Trucks and buses Recreational vehicles Terrain type: Grade	5000 0.89 1404 4 0 Level	1850 0.89 520 4 0 Level	olo	550 0.94 146 4 0 Level	vph v % %
Length Trucks and buses PCE, ET Recreational vehicle PCE, ER	mi 1.5 1.2	1.5 1.2	mi	1.5 1.2	mi

```
5847
                                             2163
                                                        609
Flow rate, vp
                                                                 pcph
                     Estimation of V12 Merge Areas
                L =
                              (Equation 13-6 or 13-7)
                ΕQ
                      -0.053 Using Equation 0
                FM
                v = v (P) = -306 pc/h
                 12 F FM
                        Capacity Checks
                                                  LOS F?
                        Actual
                                    Maximum
                        8010
                                    9000
                                                   No
    V
     FO
                        3076 pc/h
                                   (Equation 13-14 or 13-17)
    v or v
     3
          av34
               > 2700 pc/h?
Ιs
    v or v
                                    Yes
     3
         av34
                > 1.5 v /2
Ιs
    v or v
                                    Yes
          av34
                  12
     3
If yes, v
          = 2338
                                  (Equation 13-15, 13-16, 13-18, or 13-19)
       12A
                    Flow Entering Merge Influence Area
                   Actual Max Desirable
                                                   Violation?
                                4600
                   4501
                                                    No
     12A
              Level of Service Determination (if not F)
Density, D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 37.1 pc/mi/ln
                                     12
Level of service for ramp-freeway junction areas of influence E
                  Speed Estimation
Intermediate speed variable,
                                        M = 0.652
                                        S
Space mean speed in ramp influence area,
                                        S = 46.5
                                                    mph
                                        R
                                        S = 50.5
Space mean speed in outer lanes,
                                                    mph
                                        0
Space mean speed for all vehicles,
```

S = 48.2

mph

0.980

0.98

0.980

0.98

0.980

0.98

Heavy vehicle adjustment, fHV

2015 Existing Conditions

- 1. 2015 PM Diverge to I-90
- 2. 2015 PM Diverge to Route 30
- 3. 2015 PM Merge onto Route 30
- 4. 2015 PM Merge onto I-90

Phone: Fax: E-mail: _____Diverge Analysis_____ Seth Analyst: Agency/Co.: CTPS Agency/Co.:

Date performed:

Analysis time period:

PM Peak Hour Freeway/Dir of Travel: I-95 Southbound Junction: Diverge to Mass Pike Jurisdiction: Highway District 6 Analysis Year: 2015 Description: Low-cost Improvements to Bottleneck Locations _____Freeway Data_____ Type of analysis Diverge Number of lanes in freeway 4 55.0 7500 Free-flow speed on freeway mph Volume on freeway vph Off Ramp Data____ Side of freeway Right Number of lanes in ramp 1 Free-Flow speed on ramp 25.0 mph Volume on ramp 2250 vph Length of first accel/decel lane 350 ft Length of second accel/decel lane _____Adjacent Ramp Data (if one exists)__ Yes 550 Does adjacent ramp exist? Volume on adjacent ramp vph Position of adjacent ramp Downstream Off Type of adjacent ramp 1400 ft Distance to adjacent ramp _____Conversion to pc/h Under Base Conditions_____ Junction Components Freeway Ramp Adjacent Ramp
7500 2250 550 V
0.94 0.99 0.93
1995 568 148 V
4 4 4 8
0 0 0 0 8
Level Level Level
0.00 % 0.00 % 0.00 %
0.00 mi 0.00 mi 0.00 mi
1.5 1.5 1.5
1.2 1.2 1.2 Volume, V (vph) vph Peak-hour factor, PHF Peak 15-min volume, v15 Trucks and buses Recreational vehicles Terrain type:

Grade Length

Trucks and buses PCE, ET

Recreational vehicle PCE, ER

```
Driver population factor, fP
                                     0.98
                                                0.98
                                                            0.98
                                     8304
Flow rate, vp
                                                2365
                                                            616
                                                                     pcph
                      Estimation of V12 Diverge Areas
                 L =
                                (Equation 13-12 or 13-13)
                 ΕQ
                        0.436 Using Equation 0
                 FD
                 v = v + (v - v) P = 4954 pc/h
                           F R FD
                  12
                     R
                          Capacity Checks
                                                      LOS F?
                         Actual
                                       Maximum
                         8304
                                       9000
                                                      No
    v = v
     Fi F
                         5939
                                       9000
                                                      No
    v = v - v
     FO
          \mathbf{F}
            R
                         2365
                                       1900
                                                      Yes
     R
    v or v
                         1675 pc/h (Equation 13-14 or 13-17)
     3 av34
Ιs
    v or v
               > 2700 pc/h?
                                       No
     3
           av34
                > 1.5 v /2
Ts
    v or v
                                       No
     3
          av34
                       12
If yes, v = 4954
                                    (Equation 13-15, 13-16, 13-18, or 13-19)
        12A
                     Flow Entering Diverge Influence Area
                     Actual
                                  Max Desirable
                                                       Violation?
                     4954
                                  4400
                                                       Yes
    v
     12
               Level of Service Determination (if not F)
                      D = 4.252 + 0.0086 \text{ v} - 0.009 \text{ L} = 43.7 \text{ pc/mi/ln}
Density,
                      R
                                         12 D
Level of service for ramp-freeway junction areas of influence F
                 Speed Estimation
                                           D = 0.771
Intermediate speed variable,
                                           S
Space mean speed in ramp influence area,
                                           S = 45.0
                                                       mph
                                           R
Space mean speed in outer lanes,
                                           S = 57.7
                                                       mph
                                           0
```

S = 49.4

mph

0.980

0.980

0.980

Heavy vehicle adjustment, fHV

Space mean speed for all vehicles,

Phone: Fax: E-mail: _____Diverge Analysis Seth Analyst: Agency/Co.: CTPS Agency/Co.:

Date performed:

Analysis time period:

PM Peak Hour Freeway/Dir of Travel: I-95 Southbound Junction: Diverge to Route 30
Jurisdiction: Highway District 6
Analysis Year: 2015 Existing Description: Low-cost Improvements to Bottleneck Location _____Freeway Data_____ Type of analysis Diverge Number of lanes in freeway 4 55.0 5250 Free-flow speed on freeway mph Volume on freeway vph ____Off Ramp Data____ Side of freeway Right Number of lanes in ramp 1 Free-Flow speed on ramp mph 25.0 Volume on ramp 550 vph Length of first accel/decel lane 500 ft Length of second accel/decel lane _____Adjacent Ramp Data (if one exists)__ Yes 500 Does adjacent ramp exist? Volume on adjacent ramp vph Position of adjacent ramp Downstream Type of adjacent ramp On ft Distance to adjacent ramp 450 Conversion to pc/h Under Base Conditions Freeway Ramp Junction Components Adjacent vph

```
5818
Flow rate, vp
                                                610
                                                            548
                                                                     pcph
                      Estimation of V12 Diverge Areas___
                 L =
                                (Equation 13-12 or 13-13)
                 ΕQ
                        0.436 Using Equation 0
                 FD
                 v = v + (v - v) P = 2881 pc/h
                           F R FD
                  12
                     R
                          Capacity Checks
                                                      LOS F?
                         Actual
                                       Maximum
                         5818
                                       9000
                                                      No
    v = v
     Fi F
                         5208
                                       9000
                                                      No
    v = v - v
     FO
          \mathbf{F}
            R
                         610
                                       1900
                                                      No
     R
    v or v
                         1468 pc/h (Equation 13-14 or 13-17)
     3 av34
Ιs
    v or v
               > 2700 pc/h?
                                       No
     3
          av34
                > 1.5 v /2
Ts
    v or v
                                       No
     3
          av34
                       12
                                    (Equation 13-15, 13-16, 13-18, or 13-19)
If yes, v = 2881
        12A
                     Flow Entering Diverge Influence Area
                                                       Violation?
                     Actual
                                  Max Desirable
                     2881
                                  4400
                                                       No
    v
     12
               Level of Service Determination (if not F)
                      D = 4.252 + 0.0086 \text{ v} - 0.009 \text{ L} = 24.5 \text{ pc/mi/ln}
Density,
                      R
                                         12 D
Level of service for ramp-freeway junction areas of influence C
                Speed Estimation
Intermediate speed variable,
                                           D = 0.613
                                           S
Space mean speed in ramp influence area,
                                           S = 47.0
                                                       mph
                                           R
Space mean speed in outer lanes,
                                           S = 58.5
                                                       mph
                                           0
Space mean speed for all vehicles,
                                          S = 52.2
                                                       mph
```

0.990

0.98

0.990

0.98

0.990

0.98

Heavy vehicle adjustment, fHV

Phone: E-mail:	Fax:	
Merge	e Analysis	
	, indiquit	
Analyst: Seth Agency/Co.: CTPS Date performed: 6/2/2015 Analysis time period: PM Peak Hour Freeway/Dir of Travel: I-95 Southbour Junction: Merge from Rou Jurisdiction: Highway Distri Analysis Year: 2015 Existing Description: Low-cost Improvements to	ite 30 Lct 6	ion
Free	eway Data	
Type of analysis Number of lanes in freeway Free-flow speed on freeway Volume on freeway	Merge 4 55.0 4700	mph vph
On F	Ramp Data	
Side of freeway Number of lanes in ramp Free-flow speed on ramp Volume on ramp Length of first accel/decel lane Length of second accel/decel lane	Right 1 25.0 500 450	mph vph ft ft
Adjacent Ramp	Data (if one exi	sts)
Does adjacent ramp exist? Volume on adjacent Ramp Position of adjacent Ramp Type of adjacent Ramp Distance to adjacent Ramp	Yes 1800 Downstream On 850	vph ft
Conversion to pc/h	n Under Base Condi	tions
Junction Components Volume, V (vph) Peak-hour factor, PHF Peak 15-min volume, v15 Trucks and buses Recreational vehicles Terrain type: Grade	Freeway Ramp 4700 500 0.92 0.92 1277 136 4 4 0 0 Level Level	Ramp 1800 vph 0.94 479 v 4 %
Length Trucks and buses PCE, ET Recreational vehicle PCE, ER	mi 1.5 1.5 1.2 1.2	mi mi 1.5 1.2

```
5317
                                                        1993
Flow rate, vp
                                             566
                                                                 pcph
                     Estimation of V12 Merge Areas
                L =
                              (Equation 13-6 or 13-7)
                ΕQ
                     0.147 Using Equation 0
                FM
                v = v (P) = 782
                                    pc/h
                 12 F FM
                        Capacity Checks
                                                  LOS F?
                        Actual
                                    Maximum
                        5883
                                    9000
                                                   No
    V
     FO
                        2267 pc/h
                                   (Equation 13-14 or 13-17)
    v or v
     3
          av34
               > 2700 pc/h?
Ιs
    v or v
                                    No
     3
         av34
                > 1.5 v /2
Ιs
    v or v
                                    Yes
                  12
     3
          av34
If yes, v
          = 2126
                                  (Equation 13-15, 13-16, 13-18, or 13-19)
       12A
                    Flow Entering Merge Influence Area
                   Actual Max Desirable
                                                   Violation?
                   2692
                                4600
                                                    No
     12A
              Level of Service Determination (if not F)
Density, D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 23.4 pc/mi/ln
                                     12
Level of service for ramp-freeway junction areas of influence C
                  Speed Estimation
Intermediate speed variable,
                                        M = 0.356
                                        S
Space mean speed in ramp influence area,
                                        S = 50.4
                                                    mph
                                        R
Space mean speed in outer lanes,
                                        S = 51.1
                                                    mph
                                        0
Space mean speed for all vehicles,
```

S = 50.7

mph

0.980

0.98

0.980

0.98

0.980

0.98

Heavy vehicle adjustment, fHV

Fax:

Phone:

E-mail:	rax.				
Merge	e Analysis				
Analyst: Agency/Co.: Date performed: Analysis time period: Freeway/Dir of Travel: Junction: Jurisdiction: Analysis Year: Description: Low-cost Seth CTPS 6/2/2015 PM Peak Hour I-95 Southbour Merge from Mass Highway District 2015 Existing	ss Pike Lct 6	: Locatior	n		
Free	eway Data				
Type of analysis Number of lanes in freeway Free-flow speed on freeway Volume on freeway	Merge 4 55.0 5200		mph vph		
On F	Ramp Data				
Side of freeway Number of lanes in ramp Free-flow speed on ramp Volume on ramp Length of first accel/decel lane Length of second accel/decel lane	Right 1 25.0 1800 400		mph vph ft ft		
Adjacent Ramp	Data (if o	ne exists	s)		
Does adjacent ramp exist? Volume on adjacent Ramp Position of adjacent Ramp Type of adjacent Ramp Distance to adjacent Ramp	Yes 400 Upstr On 800	ream	vph ft		
-		. Conditio			
Junction Components Volume, V (vph)	Freeway	Ramp 1800)119	Adjacer Ramp 400	vph
Peak-hour factor, PHF Peak 15-min volume, v15 Trucks and buses Recreational vehicles Terrain type: Grade	0.93 1398 4 0 Level	0.93 484 4 0 Level	ᅇ	0.89 112 4 0 Level	V % %
Length Trucks and buses PCE, ET Recreational vehicle PCE, ER	mi 1.5 1.2	1.5 1.2	mi	1.5 1.2	mi

```
5820
                                             2014
Flow rate, vp
                                                        468
                                                                 pcph
                     Estimation of V12 Merge Areas
                L =
                              (Equation 13-6 or 13-7)
                ΕQ
                     -0.034 Using Equation 0
                FM
                v = v (P) = -197 pc/h
                12 F FM
                        Capacity Checks
                                                  LOS F?
                        Actual
                                    Maximum
                        7834
                                    9000
                                                  No
    V
     FO
                        3008 pc/h
                                   (Equation 13-14 or 13-17)
    v or v
          av34
     3
               > 2700 pc/h?
Ιs
    v or v
                                    Yes
     3
         av34
                > 1.5 v /2
Ιs
    v or v
                                    Yes
          av34
                  12
     3
If yes, v
          = 2328
                                  (Equation 13-15, 13-16, 13-18, or 13-19)
       12A
                    Flow Entering Merge Influence Area
                   Actual Max Desirable
                                                   Violation?
                   4342
                                4600
                                                   No
     12A
              Level of Service Determination (if not F)
Density, D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 35.9 pc/mi/ln
                                    12
Level of service for ramp-freeway junction areas of influence E
                  Speed Estimation
Intermediate speed variable,
                                        M = 0.601
                                        S
Space mean speed in ramp influence area,
                                        S = 47.2
                                                   mph
                                        R
                                        S = 50.5
Space mean speed in outer lanes,
                                                   mph
                                        0
```

S = 48.6

mph

0.980

0.98

0.980

0.98

0.980

0.98

Heavy vehicle adjustment, fHV

Driver population factor, fP

Space mean speed for all vehicles,

2025 With Improvements

- 1. 2025 AM Diverge to I-90
- 2. 2025 AM Diverge to Route 30
- 3. 2025 AM Merge onto Route 30
- 4. 2025 AM Merge onto I-90

Phone: Fax: E-mail: _____Diverge Analysis_____ Seth Analyst: Agency/Co.: CTPS Agency/co.:

Date performed:

Analysis time period:

AM Peak Hour Freeway/Dir of Travel: I-95 Southbound Junction: Diverge to Mass Pike Jurisdiction: Highway District 6 Analysis Year: 2025 Future Year Description: Low-cost Improvements to Bottleneck Location _____Freeway Data_____ Type of analysis Diverge Number of lanes in freeway 4 55.0 7450 Free-flow speed on freeway mph Volume on freeway vph Off Ramp Data_____ Side of freeway Right Number of lanes in ramp 2 Free-Flow speed on ramp 25.0 mph Volume on ramp 1750 vph Length of first accel/decel lane 1000 ft Length of second accel/decel lane 500 Adjacent Ramp Data (if one exists) Yes 950 Does adjacent ramp exist? Volume on adjacent ramp vph Position of adjacent ramp Downstream Off Type of adjacent ramp 1400 ft Distance to adjacent ramp Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp
Volume, V (vph)	7450	1750	950 vph
Peak-hour factor, PHF	0.95	0.95	0.95
Peak 15-min volume, v15	1961	461	250 v
Trucks and buses	4	4	4 %
Recreational vehicles	0	0	0 %
Terrain type:	Level	Level	Level
Grade	0.00 %	0.00 %	0.00 %
Length	0.00 mi	0.00 mi	0.00 mi
Trucks and buses PCE, ET	1.5	1.5	1.5
Recreational vehicle PCE, ER	1.2	1.2	1.2

```
Driver population factor, fP
                                     0.98
                                                0.98
                                                            0.98
                                     8162
Flow rate, vp
                                                1917
                                                            1041
                                                                     pcph
                      Estimation of V12 Diverge Areas
                 L =
                                (Equation 13-12 or 13-13)
                 ΕQ
                        0.260 Using Equation 0
                 FD
                 v = v + (v - v) P = 3541 pc/h
                           F R FD
                  12
                     R
                          Capacity Checks
                                                     LOS F?
                         Actual
                                       Maximum
                         8162
                                       9000
                                                      No
    v = v
     Fi F
                         6245
                                       9000
                                                      No
    v = v - v
     FO
          \mathbf{F}
            R
                         1917
                                       3800
                                                      No
     R
    v or v
                         2310 pc/h (Equation 13-14 or 13-17)
     3 av34
Ιs
    v or v
               > 2700 pc/h?
                                      No
     3
          av34
                > 1.5 v /2
Ts
    v or v
                                      No
     3
          av34
                       12
                                    (Equation 13-15, 13-16, 13-18, or 13-19)
If yes, v = 3541
        12A
                    Flow Entering Diverge Influence Area
                                                       Violation?
                    Actual
                                  Max Desirable
                    3541
                                  4400
                                                       No
    v
     12
               Level of Service Determination (if not F)
                     D = 4.252 + 0.0086 \text{ v} - 0.009 \text{ L} = 12.2 \text{ pc/mi/ln}
Density,
                      R
                                        12 D
Level of service for ramp-freeway junction areas of influence B
                Speed Estimation
                                          D = 0.731
Intermediate speed variable,
                                           S
Space mean speed in ramp influence area,
                                          S = 45.5
                                                       mph
                                           R
Space mean speed in outer lanes,
                                          S = 55.2
                                                       mph
                                           0
```

S = 50.5

mph

0.980

0.980

0.980

Heavy vehicle adjustment, fHV

Space mean speed for all vehicles,

Phone: Fax: E-mail: _____Diverge Analysis Seth Analyst: Agency/Co.: CTPS Agency/Co.:

Date performed:

Analysis time period:

AM Peak Hour Freeway/Dir of Travel: I-95 Southbound Junction: Diverge to Route 30
Jurisdiction: Highway District 6
Analysis Year: 2025 Future Year Description: Low-cost Improvements to Bottleneck Location ______Freeway Data_____ Type of analysis Diverge Number of lanes in freeway 3 60.0 5500 Free-flow speed on freeway mph Volume on freeway vph _____Off Ramp Data____ Side of freeway Right Number of lanes in ramp 1 Free-Flow speed on ramp mph 25.0 Volume on ramp 950 vph Length of first accel/decel lane 500 ft Length of second accel/decel lane _____Adjacent Ramp Data (if one exists)__ Yes 550 Does adjacent ramp exist? Volume on adjacent ramp vph Position of adjacent ramp Downstream Type of adjacent ramp On ft Distance to adjacent ramp 450 Conversion to pc/h Under Base Conditions_____ Freeway Ramp Junction Components Adjacent Ramp

5500 950 550 v

0.94 0.94 0.94

1463 253 146 v

4 4 4 8

0 0 0 0 8

Level Level Level

0.00 % 0.00 % 0.00 %

0.00 mi 0.00 mi 0.00 mi

1.5 1.5 1.5

1.2 1.2 1.2 Volume, V (vph) vph Peak-hour factor, PHF Peak 15-min volume, v15 Trucks and buses Recreational vehicles Terrain type: Grade

Length

Trucks and buses PCE, ET

Recreational vehicle PCE, ER

```
Driver population factor, fP
                                     0.98
                                                0.98
                                                            0.98
                                     6090
                                                            609
Flow rate, vp
                                                1052
                                                                     pcph
                      Estimation of V12 Diverge Areas
                 L =
                                (Equation 13-12 or 13-13)
                 ΕQ
                        0.559 Using Equation 9
                 FD
                 v = v + (v - v) P = 3870 pc/h
                           F R FD
                  12
                     R
                          Capacity Checks
                                                      LOS F?
                         Actual
                                       Maximum
                         6090
                                       6900
                                                      No
    v = v
     Fi F
                         5038
                                       6900
                                                      No
    v = v - v
     FO
          \mathbf{F}
            R
                         1052
                                       1900
                                                      No
     R
    v or v
                         2220 pc/h (Equation 13-14 or 13-17)
     3
          av34
Ιs
    v or v
               > 2700 pc/h?
                                       No
     3
           av34
                > 1.5 v /2
Ts
    v or v
                                       No
     3
          av34
                       12
                                    (Equation 13-15, 13-16, 13-18, or 13-19)
If yes, v = 3870
        12A
                     Flow Entering Diverge Influence Area
                                                       Violation?
                     Actual
                                  Max Desirable
                     3870
                                  4400
                                                       No
    v
     12
               Level of Service Determination (if not F)
                      D = 4.252 + 0.0086 \text{ v} - 0.009 \text{ L} = 33.0 \text{ pc/mi/ln}
Density,
                      R
                                         12 D
Level of service for ramp-freeway junction areas of influence D
                 Speed Estimation
                                           D = 0.653
Intermediate speed variable,
                                           S
Space mean speed in ramp influence area,
                                           S = 48.3
                                                       mph
                                           R
Space mean speed in outer lanes,
                                           S = 61.1
                                                       mph
                                           0
```

0.980

0.980

0.980

mph

S = 52.2

Heavy vehicle adjustment, fHV

Space mean speed for all vehicles,

Phone: E-mail:	Fax:	
Merne	Analysis	
Analyst: Seth Agency/Co.: CTPS Date performed: 6/2/2015 Analysis time period: AM Peak Hour Freeway/Dir of Travel: I-95 Southboun Junction: Merge from Rou Jurisdiction: Highway Distri Analysis Year: 2025 Future Ye Description: Low-cost Improvements to	te 30 ct 6 ar	on
Free	way Data	
Type of analysis Number of lanes in freeway Free-flow speed on freeway Volume on freeway	Merge 3 55.0 4600	mph vph
On N	.amp Daca	
Side of freeway Number of lanes in ramp Free-flow speed on ramp Volume on ramp Length of first accel/decel lane Length of second accel/decel lane	Right 1 25.0 550 450	mph vph ft ft
Adjacent Ramp	Data (if one exist	cs)
Does adjacent ramp exist? Volume on adjacent Ramp Position of adjacent Ramp Type of adjacent Ramp Distance to adjacent Ramp	Yes 1900 Downstream On 850	vph ft
Conversion to pc/h	Under Base Conditi	ons
Junction Components Volume, V (vph) Peak-hour factor, PHF Peak 15-min volume, v15 Trucks and buses Recreational vehicles Terrain type:	Freeway Ramp 4600 550 0.94 0.94 1223 146 4 4 0 0 Level Level	Adjacent Ramp 1900 vph 0.89 534 v 4 % 0 % Level
Grade Length Trucks and buses PCE, ET Recreational vehicle PCE, ER	% mi 1.5 1.5 1.2 1.2	% % mi mi 1.5 1.2

```
5093
                                             609
Flow rate, vp
                                                        2222
                                                                 pcph
                     Estimation of V12 Merge Areas
                L =
                              (Equation 13-6 or 13-7)
                ΕQ
                      0.590 Using Equation 3
                FM
                v = v (P) = 3005 pc/h
                 12 F FM
                        Capacity Checks
                                                  LOS F?
                        Actual
                                    Maximum
                        5702
                                    6750
                                                   No
    V
     FO
                        2088 pc/h
                                   (Equation 13-14 or 13-17)
    v or v
          av34
     3
               > 2700 pc/h?
Ιs
    v or v
                                    No
     3
         av34
                > 1.5 v /2
Ιs
    v or v
                                    Yes
                  12
     3
          av34
If yes, v
          = 3005
                                  (Equation 13-15, 13-16, 13-18, or 13-19)
       12A
                    Flow Entering Merge Influence Area
                   Actual Max Desirable
                                                   Violation?
                                4600
                   3614
                                                    No
     12A
              Level of Service Determination (if not F)
Density, D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 30.6 pc/mi/ln
                                     12
Level of service for ramp-freeway junction areas of influence D
                  Speed Estimation
Intermediate speed variable,
                                        M = 0.443
                                        S
Space mean speed in ramp influence area,
                                        S = 49.2
                                                    mph
                                        R
                                        S = 49.3
Space mean speed in outer lanes,
                                                    mph
                                        0
Space mean speed for all vehicles,
```

S = 49.3

mph

0.980

0.98

0.980

0.98

0.980

0.98

Heavy vehicle adjustment, fHV

Phone: E-mail:		Fax:	
	Operational Ana	lysis	
Analyst: Agency or Company: Date Performed: Analysis Time Period: Freeway/Direction: From/To: Jurisdiction: Analysis Year: Description: Low-cost	I-95 Southbound Auxiliary Lane Highway District 2025 Future Year	6	on
	Flow Inputs and	Adjustments	
Volume, V Peak-hour factor, PHF Peak 15-min volume, v15 Trucks and buses Recreational vehicles Terrain type: Grade Segment length Trucks and buses PCE, E Recreational vehicle PC Heavy vehicle adjustment Driver population factor Flow rate, vp	T E, ER t, fHV	7200 0.96 1875 4 0 Level - - 1.5 1.2 0.980 0.98 1952	veh/h v % % mi pc/h/ln
	Speed Inputs an		1 2, 22, 22
Lane width Right-side lateral clea Total ramp density, TRD Number of lanes, N Free-flow speed: FFS or BFFS Lane width adjustment, Lateral clearance adjus TRD adjustment Free-flow speed, FFS	fLW	- - 4 Measured 55.0 - - - 55.0	ft ft ramps/mi mi/h mi/h mi/h mi/h mi/h
	LOS and Perform	ance Measures	
Flow rate, vp Free-flow speed, FFS Average passenger-car s Number of lanes, N Density, D	peed, S	1952 55.0 54.4 4 35.9	<pre>pc/h/ln mi/h mi/h pc/mi/ln</pre>

E

Level of service, LOS

Overall results are not computed when free-flow speed is less than 55 mph.

2025 With Improvements

- 1. 2025 PM Diverge to I-90
- 2. 2025 PM Diverge to Route 30
- 3. 2025 PM Merge onto Route 30
- 4. 2025 PM Merge onto I-90

Phone: Fax: E-mail: _____Diverge Analysis_____ Seth Analyst: Agency/Co.: CTPS Agency/Co.:

Date performed:

Analysis time period:

PM Peak Hour Freeway/Dir of Travel: I-95 Southbound Junction: Diverge to Mass Pike Jurisdiction: Highway District 6 Analysis Year: 2025 Future Year Description: Low-cost Improvements to Bottleneck Locations _____Freeway Data_____ Type of analysis Diverge Number of lanes in freeway 4 55.0 7500 Free-flow speed on freeway mph Volume on freeway vph Off Ramp Data_____ Side of freeway Right Number of lanes in ramp 2 Free-Flow speed on ramp 30.0 mph Volume on ramp 2250 vph Length of first accel/decel lane 1000 ft Length of second accel/decel lane 500 Adjacent Ramp Data (if one exists) Yes 550 Does adjacent ramp exist? Volume on adjacent ramp vph Position of adjacent ramp Downstream Off Type of adjacent ramp 1400 ft Distance to adjacent ramp Conversion to pc/h Under Base Conditions

Junction Components	Freeway	Ramp	Adjacent Ramp
Volume, V (vph)	7500	2250	550 vph
Peak-hour factor, PHF	0.96	0.96	0.96
Peak 15-min volume, v15	1953	586	143 v
Trucks and buses	4	4	3 %
Recreational vehicles	0	0	0 %
Terrain type:	Level	Level	Level
Grade	0.00 응	0.00 %	0.00 %
Length	0.00 mi	0.00 mi	0.00 mi
Trucks and buses PCE, ET	1.5	1.5	1.5
Recreational vehicle PCE, ER	1.2	1.2	1.2

```
Driver population factor, fP
                                     0.98
                                                0.98
                                                            0.98
                                     8131
                                                            593
Flow rate, vp
                                                2439
                                                                     pcph
                      Estimation of V12 Diverge Areas
                 L =
                                (Equation 13-12 or 13-13)
                 ΕQ
                        0.260 Using Equation 0
                 FD
                 v = v + (v - v) P = 3919 pc/h
                           F R FD
                  12
                     R
                          Capacity Checks
                                                     LOS F?
                         Actual
                                       Maximum
                         8131
                                       9000
                                                      No
    v = v
     Fi F
                         5692
                                       9000
                                                      No
    v = v - v
     FO
          \mathbf{F}
            R
                         2439
                                       4000
                                                      No
     R
    v or v
                         2106 pc/h (Equation 13-14 or 13-17)
     3 av34
Ιs
    v or v
               > 2700 pc/h?
                                      No
     3
           av34
                > 1.5 v /2
Ts
    v or v
                                       No
     3
          av34
                       12
                                    (Equation 13-15, 13-16, 13-18, or 13-19)
If yes, v = 3919
        12A
                     Flow Entering Diverge Influence Area
                                                       Violation?
                     Actual
                                  Max Desirable
                     3919
                                  4400
                                                       No
    v
     12
               Level of Service Determination (if not F)
                     D = 4.252 + 0.0086 \text{ v} - 0.009 \text{ L} = 15.5 \text{ pc/mi/ln}
Density,
                      R
                                        12 D
Level of service for ramp-freeway junction areas of influence B
                Speed Estimation
                                           D = 0.713
Intermediate speed variable,
                                           S
Space mean speed in ramp influence area,
                                          S = 45.7
                                                       mph
                                           R
Space mean speed in outer lanes,
                                           S = 56.0
                                                       mph
                                           0
```

S = 50.5

mph

0.980

0.980

0.985

Heavy vehicle adjustment, fHV

Space mean speed for all vehicles,

Phone: Fax: E-mail: _____Diverge Analysis Seth Analyst: Agency/Co.: CTPS Agency/Co.:

Date performed:

Analysis time period:

PM Peak Hour Freeway/Dir of Travel: I-95 Southbound Junction: Diverge to Route 30
Jurisdiction: Highway District 6
Analysis Year: 2025 Future Year Description: Low-cost Improvements to Bottleneck Locations _____Freeway Data_____ Type of analysis Diverge Number of lanes in freeway 3 55.0 5400 Free-flow speed on freeway mph Volume on freeway vph Off Ramp Data____ Side of freeway Right Number of lanes in ramp 1 Free-Flow speed on ramp mph 25.0 Volume on ramp 550 vph Length of first accel/decel lane 500 ft Length of second accel/decel lane _____Adjacent Ramp Data (if one exists)__ Yes 500 Does adjacent ramp exist? Volume on adjacent ramp vph Position of adjacent ramp Downstream Type of adjacent ramp On ft Distance to adjacent ramp 450 Conversion to pc/h Under Base Conditions_____ Freeway Ramp Junction Components Adjacent Ramp

5400 550 500 V

0.93 0.93 0.94

1452 148 133 V

4 4 4 8

0 0 0 0 0 8

Level Level Level

0.00 % 0.00 % 0.00 %

0.00 mi 0.00 mi 0.00 mi

1.5 1.5 1.5

1.2 1.2 1.2 Volume, V (vph) vph Peak-hour factor, PHF Peak 15-min volume, v15 Trucks and buses Recreational vehicles Terrain type: Grade

Length

Trucks and buses PCE, ET

Recreational vehicle PCE, ER

```
6043
Flow rate, vp
                                                 616
                                                            554
                                                                      pcph
                      Estimation of V12 Diverge Areas__
                 L =
                                (Equation 13-12 or 13-13)
                  ΕQ
                        0.581 Using Equation 9
                  FD
                 v = v + (v - v) P = 3767 pc/h
                           F R FD
                  12
                     R
                          Capacity Checks
                                                      LOS F?
                         Actual
                                       Maximum
                          6043
                                       6750
                                                      No
    v = v
     Fi F
                          5427
                                       6750
                                                      No
    v = v - v
     FO
          \mathbf{F}
            R
                          616
                                       1900
                                                      No
     R
    v or v
                         2276 pc/h (Equation 13-14 or 13-17)
     3
          av34
Ιs
    v or v
               > 2700 pc/h?
                                       No
     3
           av34
                 > 1.5 v /2
Ts
    v or v
                                       No
     3
          av34
                       12
                                    (Equation 13-15, 13-16, 13-18, or 13-19)
If yes, v = 3767
        12A
                     Flow Entering Diverge Influence Area
                                                       Violation?
                     Actual
                                  Max Desirable
                     3767
                                  4400
                                                       No
    v
     12
               Level of Service Determination (if not F)
                      D = 4.252 + 0.0086 \text{ v} - 0.009 \text{ L} = 32.1 \text{ pc/mi/ln}
Density,
                      R
                                         12 D
Level of service for ramp-freeway junction areas of influence D
                 Speed Estimation
Intermediate speed variable,
                                           D = 0.613
                                            S
Space mean speed in ramp influence area,
                                           S = 47.0
                                                       mph
                                           R
Space mean speed in outer lanes,
                                           S = 55.4
                                                       mph
                                           0
Space mean speed for all vehicles,
                                          S = 49.9
                                                       mph
```

0.980

0.98

0.980

0.98

0.980

0.98

Heavy vehicle adjustment, fHV

Phone: Fax: E-mail: ______Merge Analysis____ Seth Analyst: Agency/Co.: CTPS Agency/Co.:

Date performed:

Analysis time period:

PM Peak Hour Freeway/Dir of Travel: I-95 Southbound Junction: Merge from Route 30
Jurisdiction: Highway District 6
Analysis Year: 2025 Future year Description: Low-cost Improvements to Bottleneck Location _____Freeway Data_____ Type of analysis Merge Number of lanes in freeway 3 55.0 4850 Free-flow speed on freeway mph Volume on freeway vph On Ramp Data____ Side of freeway Right Number of lanes in ramp 1 Free-flow speed on ramp mph 25.0 Volume on ramp 500 vph Length of first accel/decel lane 450 ft Length of second accel/decel lane _____Adjacent Ramp Data (if one exists)_____ Yes 1850 Does adjacent ramp exist? vph Volume on adjacent Ramp Position of adjacent Ramp Downstream Type of adjacent Ramp On ft Distance to adjacent Ramp 850 _____Conversion to pc/h Under Base Conditions_____ Freeway Ramp Junction Components Adjacent Ramp

4850 500 1850

0.92 0.92 0.94

1318 136 492

4 0 0 0 0

Level Level Level

% %

mi mi

1.5 1.5 1.5

1.2 1.2 1.2 Volume, V (vph) vph Peak-hour factor, PHF Peak 15-min volume, v15 Trucks and buses Recreational vehicles Terrain type: Grade Length mi Trucks and buses PCE, ET Recreational vehicle PCE, ER

```
5487
                                                        2048
Flow rate, vp
                                             566
                                                                 pcph
                     Estimation of V12 Merge Areas
                L =
                              (Equation 13-6 or 13-7)
                ΕQ
                     0.590 Using Equation 3
                FM
                v = v (P) = 3238 pc/h
                 12 F FM
                        Capacity Checks
                                                  LOS F?
                        Actual
                                    Maximum
                        6053
                                    6750
                                                  No
    V
     FO
                        2249 pc/h
                                   (Equation 13-14 or 13-17)
    v or v
     3
          av34
               > 2700 pc/h?
Ιs
    v or v
                                    No
     3
         av34
                > 1.5 v /2
Ιs
    v or v
                                    Yes
          av34
                  12
     3
If yes, v
          = 3238
                                  (Equation 13-15, 13-16, 13-18, or 13-19)
       12A
                    Flow Entering Merge Influence Area
                   Actual Max Desirable
                                                   Violation?
                   3804
                                4600
                                                    No
     12A
              Level of Service Determination (if not F)
Density, D = 5.475 + 0.00734 v + 0.0078 v - 0.00627 L = 32.1 pc/mi/ln
                                     12
Level of service for ramp-freeway junction areas of influence D
                  Speed Estimation
Intermediate speed variable,
                                        M = 0.474
                                        S
Space mean speed in ramp influence area,
                                        S = 48.8
                                                   mph
                                        R
Space mean speed in outer lanes,
                                        S = 48.7
                                                   mph
                                        0
Space mean speed for all vehicles,
```

S = 48.8

mph

0.980

0.98

0.980

0.98

0.980

0.98

Heavy vehicle adjustment, fHV

Phone: E-mail:		Fax:	
	Operational Ana	alysis	
Analyst: Agency or Company: Date Performed: Analysis Time Period: Freeway/Direction: From/To: Jurisdiction: Analysis Year: Description: Low-cost	I-95 Southbound Auxiliary Lane: Highway District 2025 Future Year	6	ion
	Flow Inputs and	d Adjustments	
Volume, V Peak-hour factor, PHF Peak 15-min volume, v15 Trucks and buses Recreational vehicles Terrain type: Grade Segment length Trucks and buses PCE, E Recreational vehicle PC Heavy vehicle adjustmen	T E, ER t, fHV	6900 0.95 1816 4 0 Level - - 1.5 1.2 0.980 0.98	veh/h v % % % mi
Flow rate, vp	1, 15	1890	pc/h/ln
	Speed Inputs ar	nd Adjustments	
Lane width Right-side lateral clea Total ramp density, TRD Number of lanes, N Free-flow speed: FFS or BFFS Lane width adjustment, Lateral clearance adjus TRD adjustment Free-flow speed, FFS	fLW	- - 4 Measured 55.0 - - - 55.0	<pre>ft ft ramps/mi mi/h mi/h mi/h mi/h mi/h mi/h</pre>
	LOS and Perform	nance Measures	
Flow rate, vp Free-flow speed, FFS Average passenger-car s Number of lanes, N Density, D	peed, S	1890 55.0 54.8 4 34.5	pc/h/ln mi/h mi/h pc/mi/ln

D

Level of service, LOS

Overall results are not computed when free-flow speed is less than 55 mph.

APPENDIX E

MassDOT Highway Division Project Development Process

Overview of the Project Development Process

Transportation decision-making is complex and can be influenced by legislative mandates, environmental regulations, financial limitations, agency programmatic commitments, and partnering opportunities. Decision-makers and reviewing agencies, when consulted early and often throughout the project development process, can ensure that all participants understand the potential impact these factors can have on project implementation. Project development is the process that takes a transportation improvement from concept through construction.

The MassDOT Highway Division has developed a comprehensive project development process which is contained in Chapter 2 of the *MassDOT Highway Division's Project Development and Design Guide*. The eight-step process covers a range of activities extending from identification of a project need, through completion of a set of finished contract plans, to construction of the project. The sequence of decisions made through the project development process progressively narrows the project focus and, ultimately, leads to a project that addresses the identified needs. The descriptions provided below are focused on the process for a highway project, but the same basic process will need to be followed for non-highway projects as well.

1. Needs Identification

For each of the locations at which an improvement is to be implemented, MassDOT leads an effort to define the problem, establishes project goals and objectives, and defines the scope of the planning needed for implementation. To that end, it has to complete a Project Need Form (PNF), which states in general terms the deficiencies or needs related to the transportation facility or location. The PNF documents the problems and explains why corrective action is needed. For this study, the information defining the need for the project will be drawn primarily, perhaps exclusively, from the present report. Also, at this point in the process, MassDOT meets with potential participants, such as the Metropolitan Planning Organization (MPO) and community members, to allow for an informal review of the project.

The PNF is reviewed by the MassDOT Highway Division district office whose jurisdiction includes the location of the proposed project. MassDOT also sends the PNF to the MPO, for informational purposes. The outcome of this step determines whether the project requires further planning, whether it is already well supported by prior planning studies, and, therefore, whether it is ready to move forward into the design phase, or whether it should be dismissed from further consideration.

2. Planning

This phase will likely not be required for the implementation of the improvements proposed in this planning study, as this planning report should constitute the outcome of this step. However, in general, the purpose of this implementation step is for the project proponent to identify issues, impacts, and approvals that may need to be obtained, so that the subsequent design and permitting processes are understood.

The level of planning needed will vary widely, based on the complexity of the project. Typical tasks include: define the existing context, confirm project need, establish goals and objectives, initiate public outreach, define the project, collect data, develop and analyze alternatives, make recommendations, and provide documentation. Likely outcomes include consensus on the project definition to enable it to move forward into environmental documentation (if needed) and design, or a recommendation to delay the project or dismiss it from further consideration.

3. Project Initiation

At this point in the process, the proponent, MassDOT Highway Division, fills out a Project Initiation Form (PIF) for each improvement, which is reviewed by its Project Review Committee (PRC) and the MPO. The PRC is composed of the Chief Engineer, each District Highway Director, and representatives of the Project Management, Environmental, Planning, Right-of-Way, Traffic, and Bridge departments, and the MassDOT Federal Aid Program Office (FAPO). The PIF documents the project type and description, summarizes the project planning process, identifies likely funding and project management responsibility, and defines a plan for interagency and public participation. First the PRC reviews and evaluates the proposed project based on the MassDOT's statewide priorities and criteria. If the result is positive, MassDOT Highway Division moves the project forward to the design phase, and to programming review by the MPO. The PRC may provide a Project Management Plan to define roles and responsibilities for subsequent steps. The MPO review includes project evaluation based on the MPO's regional priorities and criteria. The MPO may assign project evaluation criteria score, a Transportation Improvement Program (TIP) year, a tentative project category, and a tentative funding category.

4. Environmental Permitting, Design, and Right-of-Way Process

This step has four distinct but closely integrated elements: public outreach, environmental documentation and permitting (if required), design, and right-of-way acquisition (if required). The outcome of this step is a fully designed and permitted project ready for construction. However, a project does not have to be fully designed in order for the MPO to program it in the TIP. The sections below provide more detailed information on the four elements of this step of the project development process.

Public Outreach

Continued public outreach in the design and environmental process is essential to maintain public support for the project and to seek meaningful input on the design elements. The public outreach is often in the form of required public hearings, but can also include less formal dialogues with those interested in and affected by a proposed project.

Environmental Documentation and Permitting

The project proponent, in coordination with the Environmental Services section of the MassDOT Highway Division, will be responsible for identifying and complying with all applicable federal, state, and local environmental laws and requirements. This includes determining the appropriate project category for both the Massachusetts Environmental Protection Act (MEPA) and the National Environmental Protection Act (NEPA). Environmental documentation and permitting is often completed in conjunction with the **Preliminary Design** phase described below.

Design

There are three major phases of design. The first is **Preliminary Design**, which is also referred to as the 25-percent submission. The major components of this phase include full survey of the project area, preparation of base plans, development of basic geometric layout, development of preliminary cost estimates, and submission of a functional design report. Preliminary Design, although not required to, is often completed in conjunction with the Environmental Documentation and Permitting. The next phase is **Final Design**, which is also referred to as the 75-percent and 100-percent submission. The major components of this phase include preparation of a subsurface exploratory plan (if required), coordination of utility relocations, development of traffic management plans through construction zones, development of final cost estimates, and refinement and finalization of the construction plans. Once Final Design is complete, a full set of **Plans, Specifications, and Estimates (PS&E)** is developed for the project.

Right-of-Way Acquisition

A separate set of Right-of-Way plans are required for any project that requires land acquisition or easements. The plans must identify the existing and proposed layout lines, easements, property lines, names of property owners, and the dimensions and areas of estimated takings and easements.

5. Programming (Identification of Funding)

Programming, which typically begins during the design phase, can actually occur at any time during the process, from planning to design. In this step, which is distinct from project initiation, the proponent requests that the MPO place the project in the region's Transportation Improvement Program (TIP). The proponent requesting the project's listing on the TIP can be the community or it can be one of the MPO member agencies (the Regional Planning Agency, MassDOT, and the Regional Transit Authority). The MPO then considers the project in terms of state and regional needs, evaluation criteria, and compliance with the regional Transportation Plan and decides whether to place it in the draft TIP for public review and then in the final TIP.

6. Procurement

Following project design and programming of a highway project, the MassDOT Highway Division publishes a request for proposals. It then reviews the bids and awards the contract to the qualified bidder with the lowest bid.

7. Construction

After a construction contract is awarded, MassDOT Highway Division and the contractor develop a public participation plan and a management plan for the construction process.

8. Project Assessment

The purpose of this step is to receive constituents' comments on the project development process and the project's design elements. MassDOT Highway Division can apply what is learned in this process to future projects.

Project Development Schematic Timetable

Description	Schedule Influence	Typical Duration
Step I: Problem/Need/Opportunity Identification The proponent completes a Project Need Form (PNF). This form is then reviewed by the MassDOT District office which provides guidance to the proponent on the subsequent steps	The Project Need Form has been developed so that it can be prepared quickly by the proponent, including any supporting data that is readily available. The District office shall return comments	1 to 3 months
Step II: Planning Project planning can range from agreement that the problem should be addressed through a clear solution to a detailed analysis of alternatives and their impacts.	to the proponent within one month of PNF submission. For some projects, no planning beyond preparation of the Project Need Form is required. Some projects require a planning study centered on specific project issues associated with the	Project Planning Report: 3 to 24+ months
	proposed solution or a narrow family of alternatives. More complex projects will likely require a detailed alternatives analysis.	
Step III: Project Initiation The proponent prepares and submits a Project Initiation Form (PIF) and a Transportation Evaluation Criteria (TEC) form in this step. The PIF and TEC are informally reviewed by the Metropolitan Planning Organization (MPO) and MassDOT District office, and formally reviewed by the PRC.	The PIF includes refinement of the preliminary information contained in the PNF. Additional information summarizing the results of the planning process, such as the Project Planning Report, are included with the PIF and TEC. The schedule is determined by PRC staff review (dependent on project complexity) and meeting schedule.	1 to 4 months
Step IV: Design, Environmental, and Right of Way The proponent completes the project design. Concurrently, the proponent completes necessary environmental permitting analyses and files applications for permits. Any right of way needed for the project is identified and the acquisition process begins.	The schedule for this step is dependent upon the size of the project and the complexity of the design, permitting, and right-of-way issues. Design review by the MassDOT district and appropriate sections is completed in this step.	3 to 48+ months
Step V: Programming The MPO considers the project in terms of its regional priorities and determines whether or not to include the project in the draft Regional Transportation Improvement Program (TIP) which is then made available for public comment. The TIP includes a project description and funding source.	The schedule for this step is subject to each MPO's programming cycle and meeting schedule. It is also possible that the MPO will not include a project in its Draft TIP based on its review and approval procedures.	3 to 12+ months
Step VI: Procurement The project is advertised for construction and a contract awarded.	Administration of competing projects can influence the advertising schedule.	1 to 12 months
Step VII: Construction The construction process is initiated including public notification and any anticipated public involvement. Construction continues to project completion.	The duration for this step is entirely dependent upon project complexity and phasing.	3 to 60+ months
Step VIII: Project Assessment The construction period is complete and project elements and processes are evaluated on a voluntary basis. Source: Mass DOT Highway Division Project Days	The duration for this step is dependent upon the proponent's approach to this step and any follow-up required.	1 month

Source: MassDOT Highway Division Project Development and Design Guide